# DRILLED PIER WALL PROJECT EMMONS ROAD

### TURTLECREEK TOWNSHIP WARREN COUNTY, OHIO

### SHEET NUMBER

TITLE SHEET
PIER WALL PLAN & PROFILE SHEET
DETAIL & NOTES SHEET
BORING LOGS SHEET
DETOUR SHEET

 $\begin{array}{c} 1 \\ 2 \\ 3 \end{array}$ 

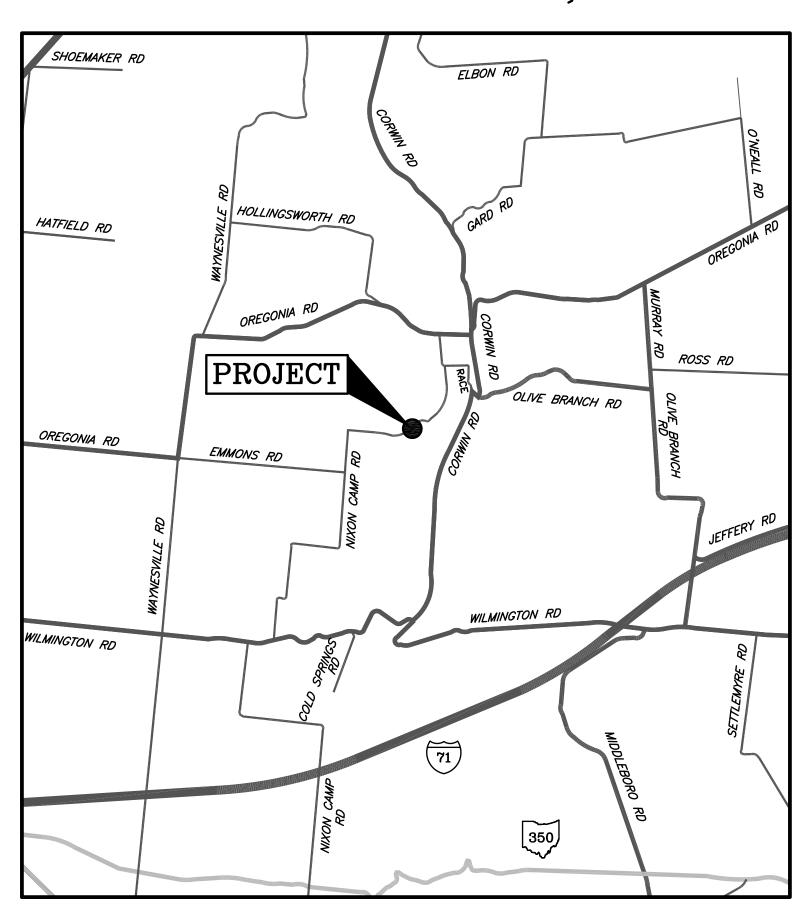
UNDERGROUND UTILITIES

2 WORKING DAYS
BEFORE YOU DIG

NON-MEMBERS MUST BE CALLED DIRECTLY

CALL TOLL FREE 800-362-2764

OHIO UTILITIES PROTECTION SERVICE



VICINITY MAP NOT TO SCALE

APPROVED:			
DATE:			
APPROVED:			
DATE:			
APPROVED:			
DATE:	WARREN	COUNTY	COMMISSIONER
APPROVED:			
DATE:	WARREN	COUNTY	COMMISSIONER

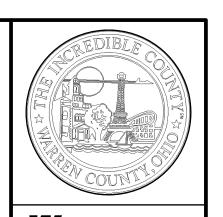


PLANS PREPARED BY:

THE WARREN COUNTY ENGINEERING DEPARTMENT

210 W MAIN STREET

LEBANON, OHIO 45036



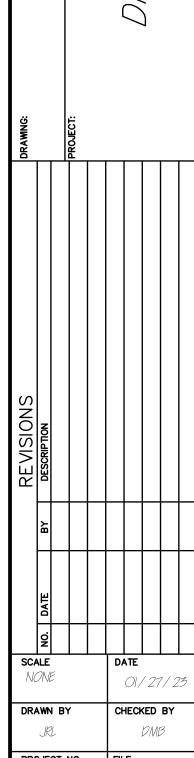
# Warren County Engineer's Office

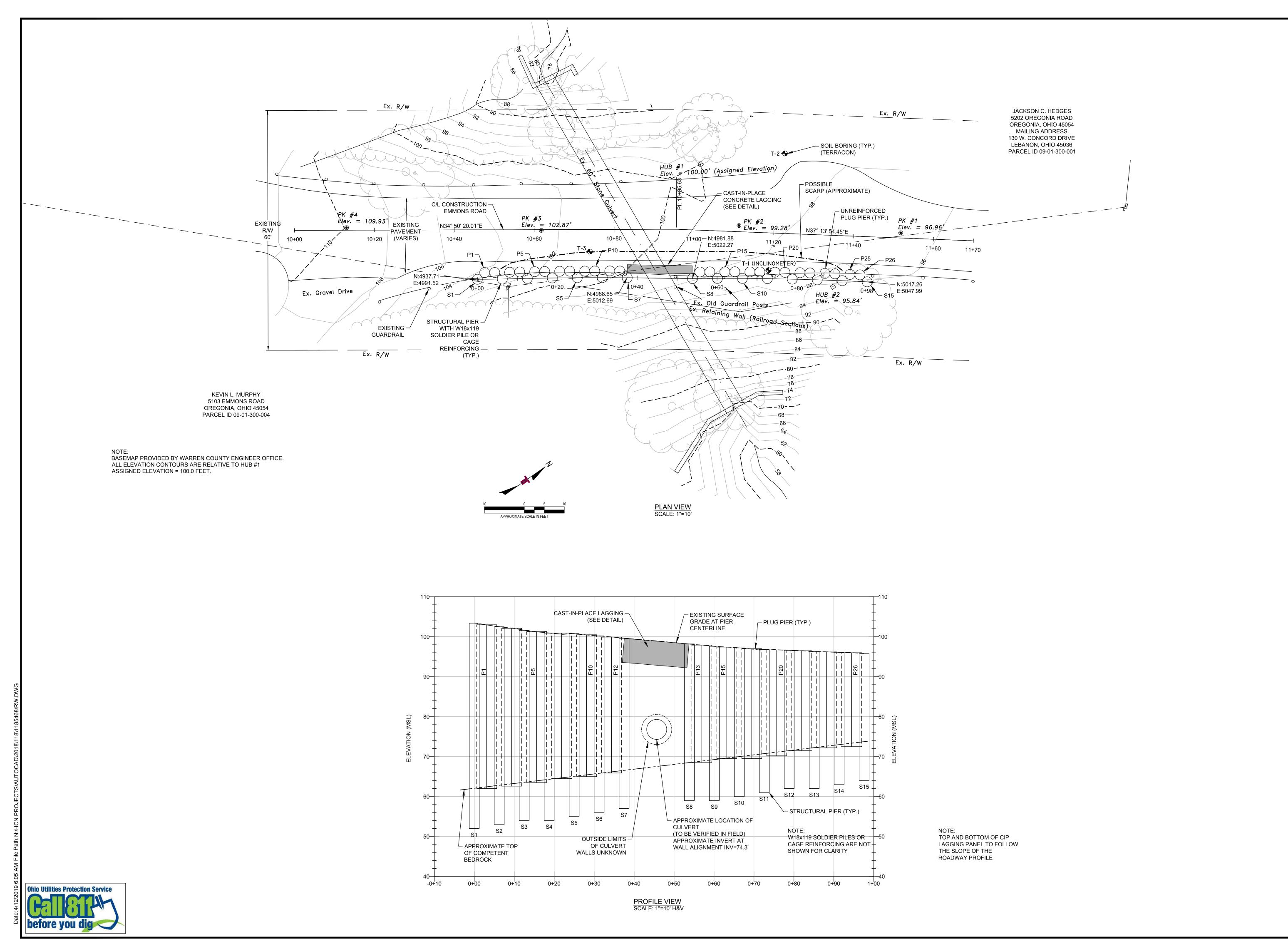
Neil F. Tunison, P.E., P.S.

Warren County Engineer

210 W Main Street
Lebanon, Ohio 45036
513 695 3301 Phone
513 695 7714 Fax

EMMONS ROAD
LED PIER WALL PROJECT





REV. DATE BY DESCRIPTION

PLAN AND PROFILE

EMMONS ROAD LANDSLIDE - WARREN COUNTY

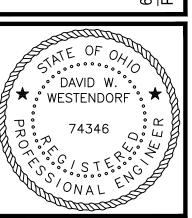
TURTLECREEK TOWNSHIP

5103 EMMONS ROAD

Consulting Engineers and Scientists

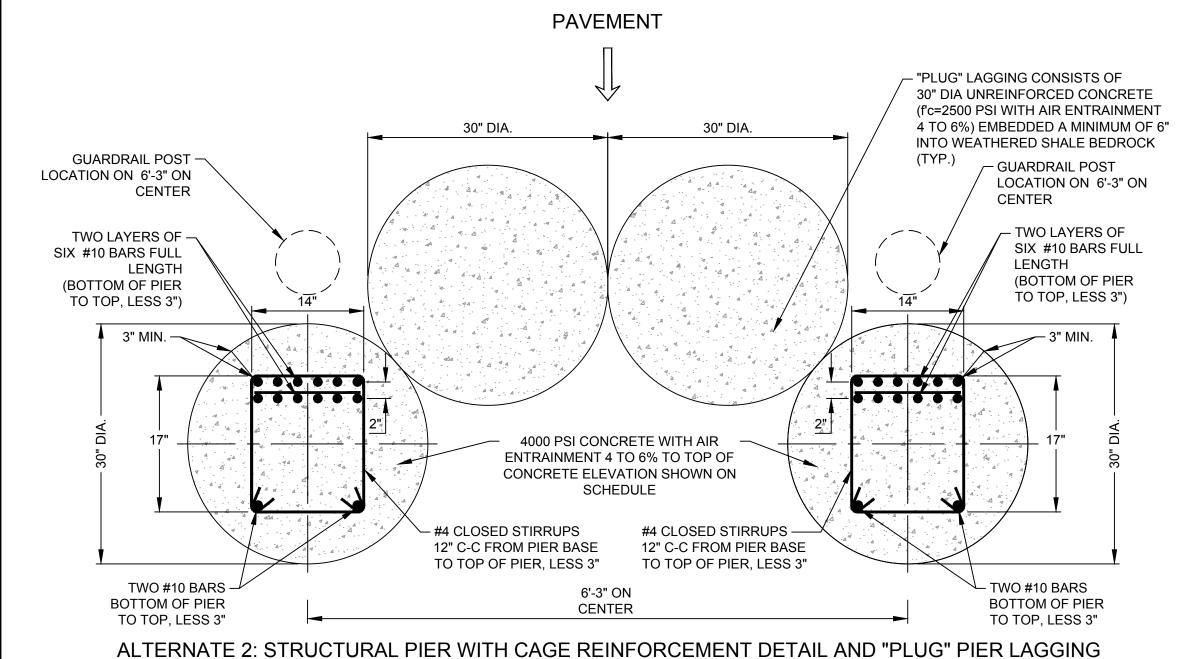
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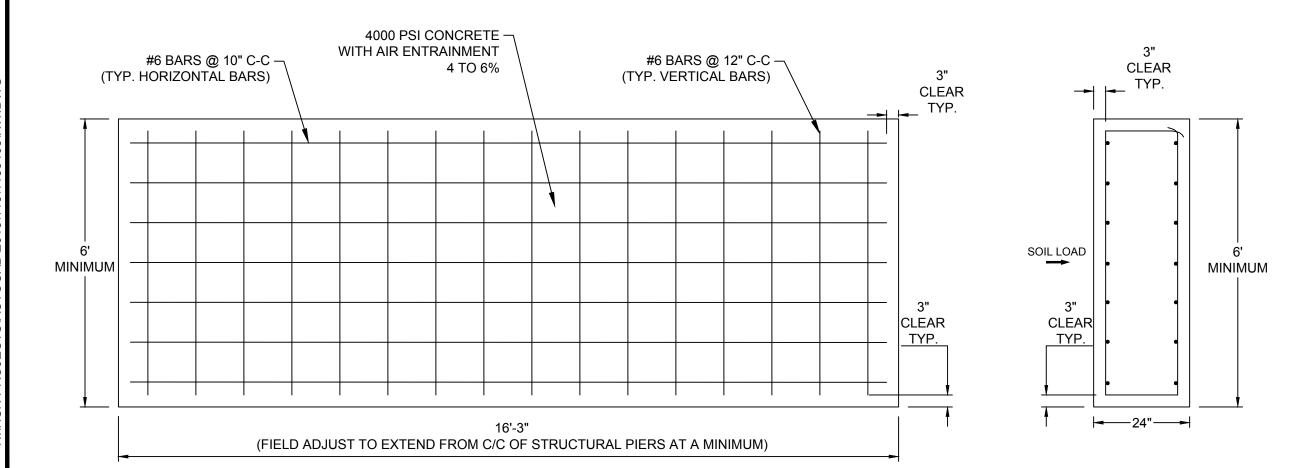
CINCINNATI, OHIO 45226



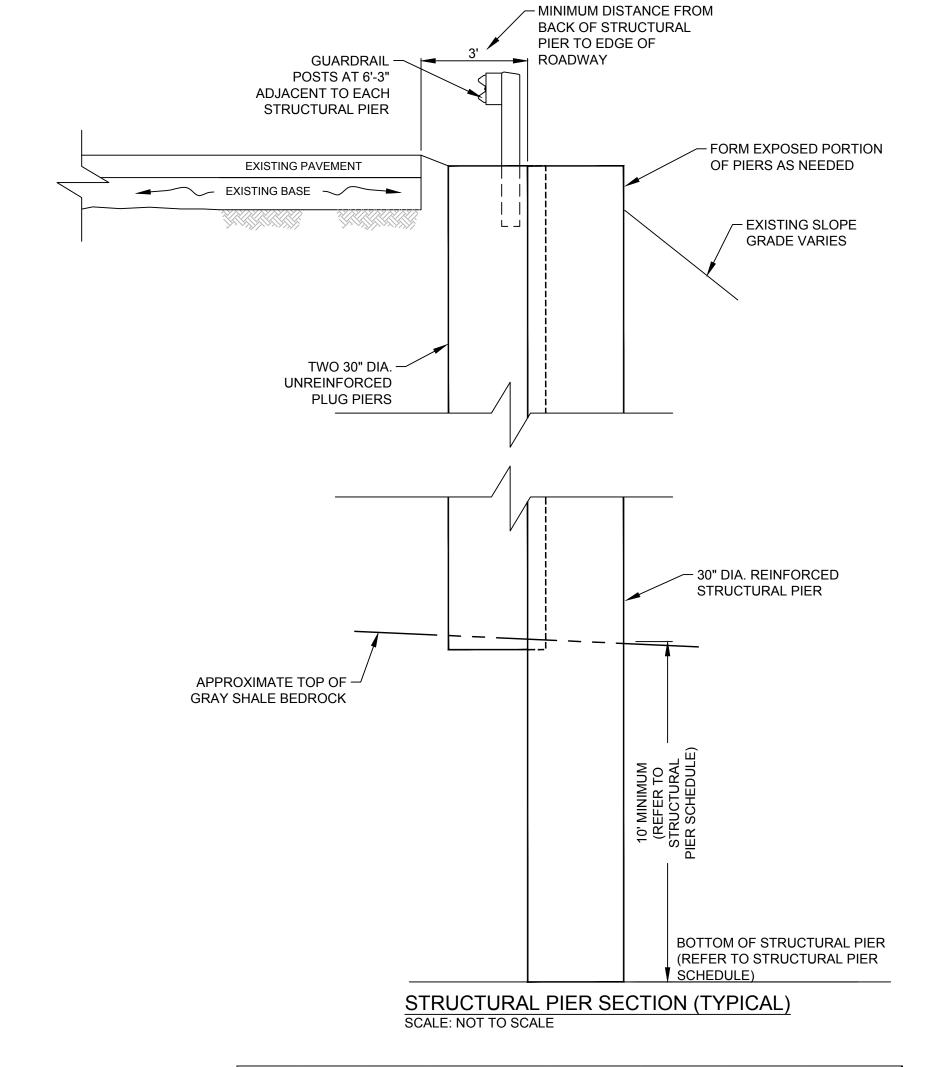
SHEET 1		
DESIGNED BY:	SK/DWW	
DRAWN BY:	KM	
APPVD. BY:	DWW	
SCALE:	AS SHOWN	
DATE:	02/26/2019	
JOB NO.	N1185468	
ACAD NO.	RW.DWG	
SHEET NO.:	1	

ALTERNATE 1: STRUCTURAL PIER WITH W18x119 REINFORCEMENT DETAIL AND "PLUG" PIER LAGGING SCALE: 1"=1'





CAST-IN-PLACE CONCRETE LAGGING DETAIL (BETWEEN S7 & S8)



	STRUCTURAL PIER SCHEDULE						
Pier No.	Diameter	Station	Approximate Top of Pier Concrete Elevation*	Approx. Top of Competent Bedrock Elevation	Minimum Rock Socket Length	Estimated Drilled Pier Bottom Elevation	Estimated Drilled Pier Length
	inches	feet	feet	feet	feet	feet	feet
S1	30	0+00	103.4	62	10	52	51.4
S2	30	0+06.25	102.5	63	10	53	49.5
S3	30	0+12.5	101.6	64	10	54	47.6
S4	30	0+18.75	100.9	64	10	54	46.9
S5	30	0+25	100.8	65	10	55	45.8
S6	30	0+31.25	100.2	66	10	56	44.2
S7	30	0+37.5	99.6	67	10	57	42.6
S8	30	0+53.75	98.2	69	10	59	39.2
S9	30	0+60	97.6	69	10	59	38.6
S10	30	0+66.25	97.2	70	10	60	37.2
S11	30	0+72.5	96.8	71	10	61	35.8
S12	30	0+78.75	96.6	72	10	62	34.6
S13	30	0+85	96.3	72	10	62	34.3
S14	30	0+91.25	96.1	73	10	63	33.1
S15	30	0+97.5	95.8	74	10	64	31.8

\*\* ACTUAL LENGTH TO BE DETERMINED BY CONDITIONS IN FIELD

Pier No.	Diameter	Approximate Top of Pier Concrete Elevation*	Estimated Drilled Pier Bottom Elevation	Estimated Drilled Pier Length**
	inches	feet	feet	feet
P1	30	103.0	62.0	41.0
P2	30	103.0	62.0	41.0
P3	30	102.1	62.6	39.5
P4	30	102.1	62.6	39.5
P5	30	101.3	63.5	37.8
P6	30	101.3	63.5	37.8
P7	30	100.9	64.5	36.4
P8	30	100.9	64.5	36.4
P9	30	100.5	65.0	35.5
P10	30	100.5	65.0	35.5
P11	30	99.9	65.9	34.0
P12	30	99.9	65.9	34.0
P13	30	97.9	68.5	29.4
P14	30	97.9	68.5	29.4
P15	30	97.4	69.5	27.9
P16	30	97.4	69.5	27.9
P17	30	97.0	69.5	27.5
P18	30	97.0	69.5	27.5
P19	30	96.7	70.2	26.5
P20	30	96.7	70.2	26.5
P21	30	96.5	71.5	25.0
P22	30	96.5	71.5	25.0
P23	30	96.2	72.2	24.0
P24	30	96.2	72.2	24.0
P25	30	96.0	72.5	23.5
P26	30	96.0	72.5	23.5
* TOP OF PIER CONCRETE TO BE APPROXIMATELY 4" BELOW PAVEMENT GRADE				

**DRILLED PIER CONSTRUCTION NOTES** 

#### DRILLED PIER INSTALLATION

- 1. CONSTRUCT THE 30-INCH DRILLED STRUCTURAL PIER RETAINING WALL WITH PLUG PIERS USING EITHER ROLLED STEEL SECTIONS OR CAGE REINFORCEMENT AS SHOWN AND DESCRIBED ON PLANS. THE PURPOSE OF THIS WORK IS TO STOP LATERAL CREEP-TYPE MOVEMENT ON THE DOWNSLOPE (SOUTHEAST) SIDE OF EMMONS ROAD.
- 2. CONTRACTOR IS RESPONSIBLE FOR CLEARING OVERHEAD AND UNDERGROUND UTILITIES AND PROVIDING ACCESS FOR
- 3. THE REINFORCED DRILLED PIER RETAINING WALL WILL CONSIST OF STRUCTURAL DRILLED PIERS SPACED APPROXIMATELY 6 FEET -3 INCH (6'-3") ON CENTER, AS SHOWN ON THE SITE PLAN DRAWING (SHEET 1). APPROXIMATE EMBEDMENT DEPTHS INTO SHALE AND LIMESTONE BEDROCK ARE SHOWN ON THE STRUCTURAL PIER SCHEDULE TABLE ON THIS SHEET; HOWEVER, ACTUAL EMBEDMENT DEPTHS WILL BE BASED ON ACTUAL FIELD CONDITIONS AS DETERMINED BY THE GEOTECHNICAL CONSULTANT (OR GEOTECHNICAL REPRESENTATIVE).
- 4. THE PIERS SHALL BE LOCATED AS SHOWN ON PLAN WITHIN 6" OF PLAN LOCATION. THE PIERS SHALL MAINTAIN A PLUMBNESS DEVIATION OF A MAXIMUM OF 1" IN 12 FT. VERTICAL HEIGHT. A MINIMUM 3" CONCRETE COVER BETWEEN THE REINFORCING STEEL AND THE EXTERIOR (SIDES AND TOP) OF THE DRILLED PIER SHALL BE PROVIDED.
- 5. TWO ALTERNATIVES FOR REINFORCEMENT OF STRUCTURAL PIERS HAVE BEEN PROVIDED. ALL REINFORCEMENT FOR THIS PROJECT SHOULD BE EPOXY COATED. REINFORCEMENT FOR THE STRUCTURAL PIERS SHALL CONSIST EITHER OF ROLLED STEEL SECTIONS HAVING YIELD STRENGTH OF 50 KSI OR CAGE REINFORCING STEEL HAVING A YIELD STRENGTH OF 60 KSI. ROLLED STEEL SECTIONS SHOULD CONSIST OF W 18X119 FOR ALTERNATIVE 1. THE CONFIGURATION OF THE REINFORCING CAGE FOR ALTERNATIVE 2 HAS BEEN SHOWN ON SHEET 2 AND CONSISTS OF A 14"X17" RECTANGULAR CAGE WITH A DOUBLE LAYER OF (6) #10 BARS, SPACED AT 2-INCHES CENTER-TO-CENTER.
- 6. THE PRE-DRILLED PIER WILL BE BACKFILLED WITH CONCRETE TO THE TOP OF CONCRETE ELEVATION. THE TOP OF PIER CONCRETE ELEVATION SHALL BE ABOUT 4 INCHES BELOW THE TOP OF PAVEMENT ELEVATION, TO AVOID CONTACT BY SNOW PLOWS. STRUCTURAL PIER CONCRETE SHALL BE CONCRETE (F'C = 4000 PSI, MAXIMUM SLUMP = 4 INCHES) WITH 4 TO 6% AIR-ENTRAINMENT PLACED USING FREE FALL METHOD OF PLACEMENT. CONCRETE SHALL BE PLACED INTO EACH PIER EXCAVATION ON THE SAME DAY THAT THE DRILLING IS COMPLETED. SEE ITEM 9 BELOW.
- 7. TEMPORARY STEEL CASING SHOULD BE ON-SITE AND USED WHEREVER REQUIRED TO STABILIZE LOOSE OR CAVING MATERIALS, OR TO SEAL OFF WATER BEARING ZONES ENCOUNTERED DURING CONSTRUCTION.
- 8. THE WALL DESIGN IS BASED ON MAXIMUM DEPTH TO COMPETENT BEDROCK (GRAY UNWEATHERED SHALE AND LIMESTONE) 39 FEET BELOW GRADE, AS NOTED ON PLAN. HOWEVER, IF BEDROCK IS ENCOUNTERED MORE THAN 2.0 FT. BELOW PLAN ELEVATION, TERRACON SHALL BE NOTIFIED IMMEDIATELY TO REVIEW AND PROVIDE ADDITIONAL RECOMMENDATIONS. LONGER OR ADDITIONAL REINFORCING MAY BE REQUIRED IN THIS CASE. EACH STRUCTURAL PIER SHALL BE SOCKETED INTO COMPETENT SHALE AND LIMESTONE BEDROCK (WEATHERED AND UNWEATHERED). A MINIMUM OF 10 FEET, AS DESCRIBED ON THE PLANS.
- 9. THE CONTRACTOR SHALL MAINTAIN A RECORD OF EACH PIER DRILLED, WHICH WILL INCLUDE AS A MINIMUM: PIER NUMBER; GROUND ELEVATION; PIER TOP ELEVATION; TOP OF BROWN WEATHERED SHALE AND LIMESTONE ELEVATIONS; TOP OF GRAY SHALE AND LIMESTONE ELEVATIONS; AS-BUILT ROCK SOCKET DEPTH; ELEVATION OF THE TOP OF THE PIER CONCRETE; DATE DRILLED; DATE COMPLETED; AND WEATHER CONDITIONS.
- 10. IT IS ANTICIPATED THAT WATER MAY ENTER SOME OF THE PIER EXCAVATIONS. THE DEPTH OF PONDED WATER AT THE BOTTOM OF THE PIER EXCAVATIONS SHOULD NOT EXCEED 2 INCHES, PRIOR TO PLACING CONCRETE. IF THE WATER CANNOT BE PUMPED DOWN, TREMIE PLACEMENT METHODS WILL BE REQUIRED.
- 11. THE DRILLED PIER EXCAVATIONS SHOULD BE INSPECTED BY A QUALIFIED GEOTECHNICAL REPRESENTATIVE TO CONFIRM THAT THE DRILLED PIERS ARE SOCKETED INTO BEDROCK ACCORDING TO DESIGN, AND THAT THE DRILLED PIERS HAVE BEEN CONSTRUCTED PER SPECIFICATIONS.
- 12. PIER SPOILS SHALL BE REMOVED FROM THE SITE (NOT WASTED ON THE HILLSIDE). NO FILL PLACEMENT SHOULD BE ALLOWED DOWNSLOPE OF THE SLOPE FACE.

#### PLUG PIER INSTALLATION

- 1. PAIRS OF 30-INCH DIAMETER PLUG PIERS CONSISTING OF UNREINFORCED CONCRETE (F'C = 2500 PSI, MAXIMUM SLUMP = 4 INCHES) WITH 4 TO 6% AIR ENTRAINMENT AS NOTED IN THE CROSS-SECTION DETAILS WILL ACT AS LAGGING FOR THE PIER
- 2. PLUG PIER INSTALLATION FOR PIER WALL SHALL BEGIN AFTER THE STRUCTURAL PIER ELEMENTS HAVE GAINED STRENGTH (A' LEAST 24 HOURS AFTER PLACEMENT OF STRUCTURAL PIER CONCRETE).
- 3. THE TOP OF THE PLUG PIER CONCRETE SHALL BE ABOUT 4 INCHES BELOW THE PAVEMENT.
- 4. THE BOTTOM OF ALL THE PLUG PIERS INSTALLED SHOULD EXTEND TO THE ELEVATIONS DETAILED IN THE DRILLED PIER SCHEDULE (A MINIMUM OF 6 INCHES BELOW TOP OF WEATHERED OR UNWEATHERED BEDROCK).
- 5. PLUG PIER SPOILS SHALL BE TRUCKED FROM THE SITE (NOT WASTED ON THE HILLSIDE).

#### **CAST-IN PLACE LAGGING**

- DUE TO THE PRESENCE OF AN EXISTING 60-INCH DIAMETER STONE CULVERT LOCATED BETWEEN STRUCTURAL PIERS S-7 AND S-8, CAST-IN PLACE LAGGING WILL BE REQUIRED TO PREVENT DAMAGE TO THE CULVERT, REDUCE EROSION, AND LOSS OF MATERIALS IN THIS AREA. THE LAGGING WILL NOT PREVENT LATERAL SOIL MOVEMENT BETWEEN STRUCTURAL PIERS S-7 AND
- 2. THE EXACT LOCATION AND THICKNESS OF THE CULVERT IS UNKNOWN. THE LOCATION OF THE CULVERT SHOULD BE CONFIRMED BY THE CONTRACTOR AND THE PIER LAYOUT ADJUSTED BASED ON THIS LOCATION.
- 3. THE CAST-IN-PLACE LAGGING SHALL CONSIST OF A 24-INCH WIDE EXCAVATION A MINIMUM OF 6 FEET DEEP, EXTENDING FROM AT LEAST THE CENTER OF STRUCTURAL PIER S-7 TO STRUCTURAL PIER S-8.
- 4. REINFORCING STEEL SHALL BE PLACED PER THE INCLUDED DETAIL AND CONSIST OF 60 KSI STEEL. ALL REINFORCING STEEL FOR THIS PROJECT SHOULD BE EPOXY COATED. THE CONCRETE SHALL BE MINIMUM F'C =4000 PSI WITH 4 TO 6% AIR ENTRAINMENT.

#### **CONSTRUCTION CONSIDERATIONS**

- 1. NEW STRUCTURAL FILL SHALL BE PLACED ON UPSLOPE SIDE OF PIERS TO RETAIN GRADE NEXT TO THE EDGE OF PAVEMENT. SONOTUBES OR EQUIVALENT WILL BE REQUIRED IN SOME AREAS, DUE TO THE STEEPLY SLOPING HILLSIDE BELOW THE WALL FILL SHOULD BE PLACED AND COMPACTED PER ODOT SPECIFICATIONS (ITEM 203). ONLY HAND-OPERATED EQUIPMENT SHOULD BE USED WITHIN 5 FEET OF THE FRONT OF THE PIERS.
- 2. EXISTING GUARDRAIL WILL BE REMOVED AND REPLACED IN THIS PROJECT.

#### FIELD QUALITY CONTROL

- A. OWNER WILL COORDINATE FIELD CONSTRUCTION INSPECTION AND REPORTING THROUGH IN-HOUSE PERSONNEL OF EXTERNAL TESTING AGENCY.
- DOCUMENTATION SHALL INCLUDE THE FOLLOWING AT EACH PIER (AND CAST-IN-PLACE LAGGING AS NOTED) GROUND ELEVATION
- 2. AS-BUILT PIER DIAMETER AND TOP AND BOTTOM PIER ELEVATIONS
- 3. TOP OF WEATHERED BROWN SHALE ELEVATION.
- TOP OF GRAY SHALE ELEVATION.
- DESCRIPTION OF ENCOUNTERED SOIL MATERIALS.
- 6. DESCRIPTION, LOCATION, AND DIMENSIONS OF OBSTRUCTIONS.
- 7. FINAL TOP CENTERLINE LOCATION AND DEVIATIONS FROM REQUIREMENTS.
- 8. VARIATION OF PIER FROM PLUMB.
- DRILLED PIER EXCAVATING METHOD.
- 10. LENGTH OF ROCK SOCKET.
- 11. LEVELNESS OF PIER BOTTOM AND ADEQUACY OF CLEANOUT.
- 12. GROUND-WATER CONDITIONS AND WATER-INFILTRATION RATE, DEPTH, AND PUMPING
- 13. DESCRIPTION, DIAMETER, AND TOP AND BOTTOM ELEVATIONS OF TEMPORARY OR PERMANENT CASINGS.
- 14. DESCRIPTION OF SOIL OR WATER MOVEMENT, SIDEWALL STABILITY, LOSS OF GROUND, AND MEANS OF CONTROL
- 15. DATE AND TIME OF STARTING AND COMPLETING DRILLED PIER EXCAVATION.
- 16. POSITION OF REINFORCING STEEL, INCLUDING IN CAST-IN-PLACE LAGGING.
- 17. CONCRETE PLACEMENT METHOD, INCLUDING DELAYS, INCLUDING IN CAST-IN-PLACE LAGGING.
- 18. ELEVATION OF CONCRETE DURING REMOVAL OF CASINGS, INCLUDING IN CAST-IN-PLACE LAGGING.
- 19. LOCATIONS OF CONSTRUCTION JOINTS, IF ANY, INCLUDING IN CAST-IN-PLACE LAGGING. 20. REMARKS, UNUSUAL CONDITIONS ENCOUNTERED, AND DEVIATIONS FROM REQUIREMENTS.

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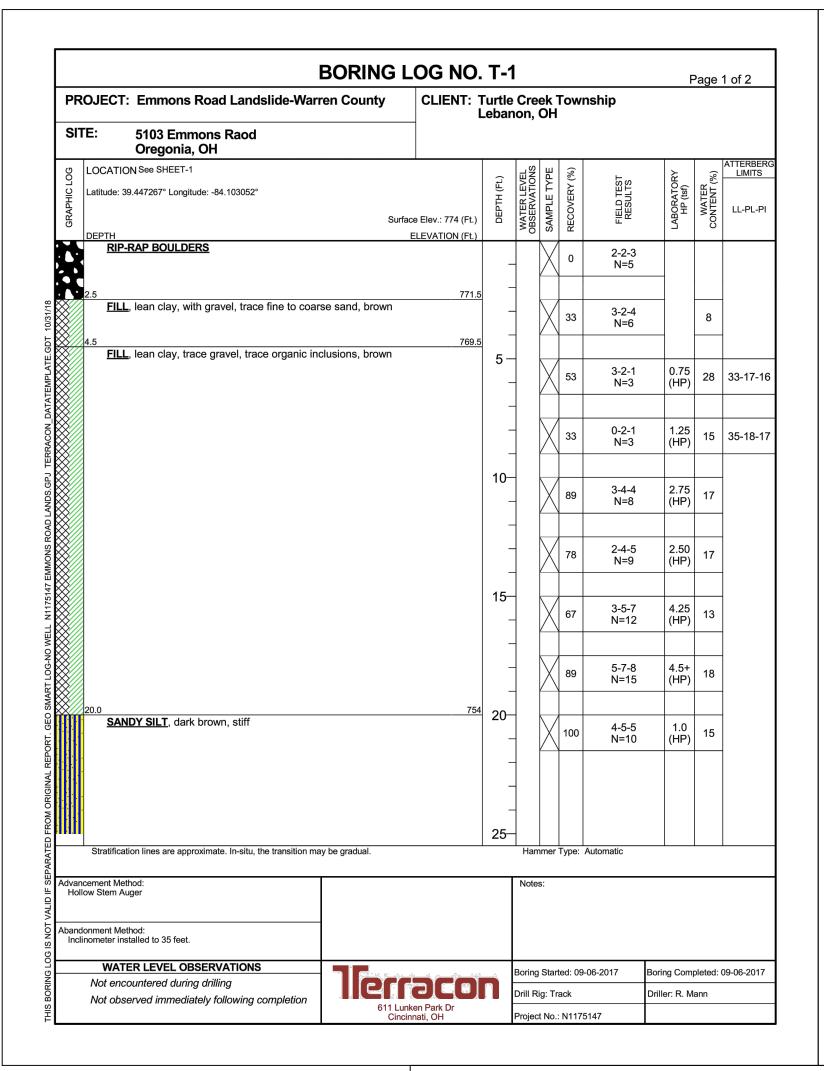
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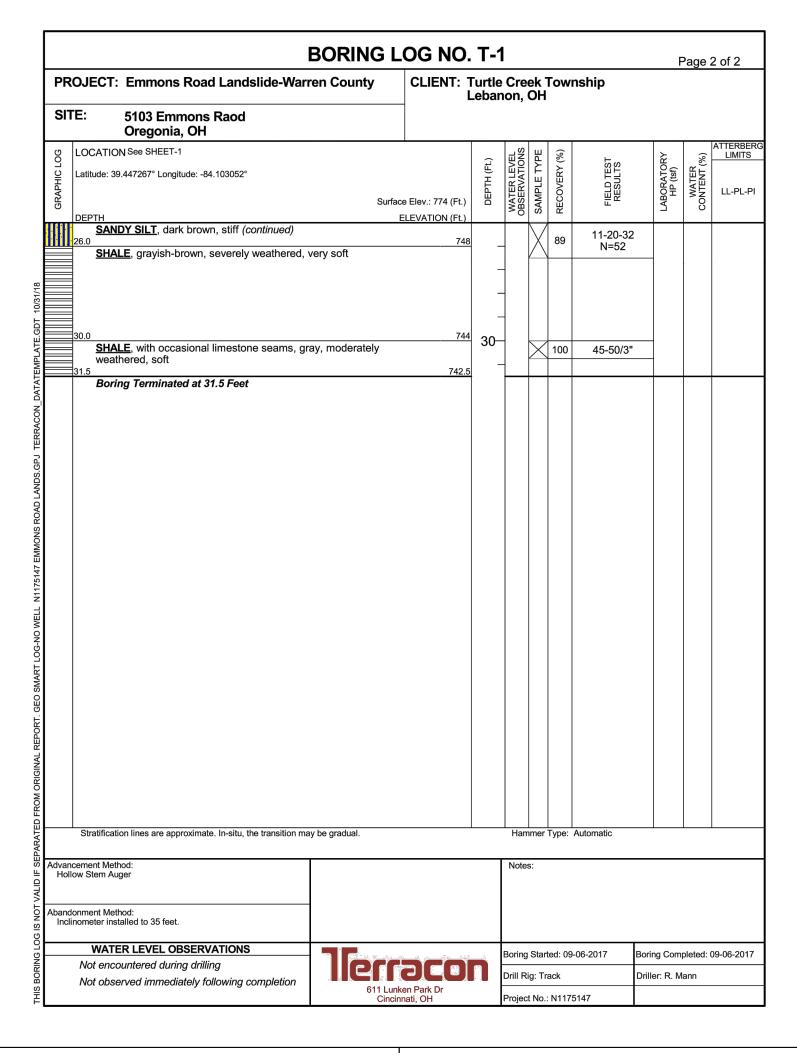
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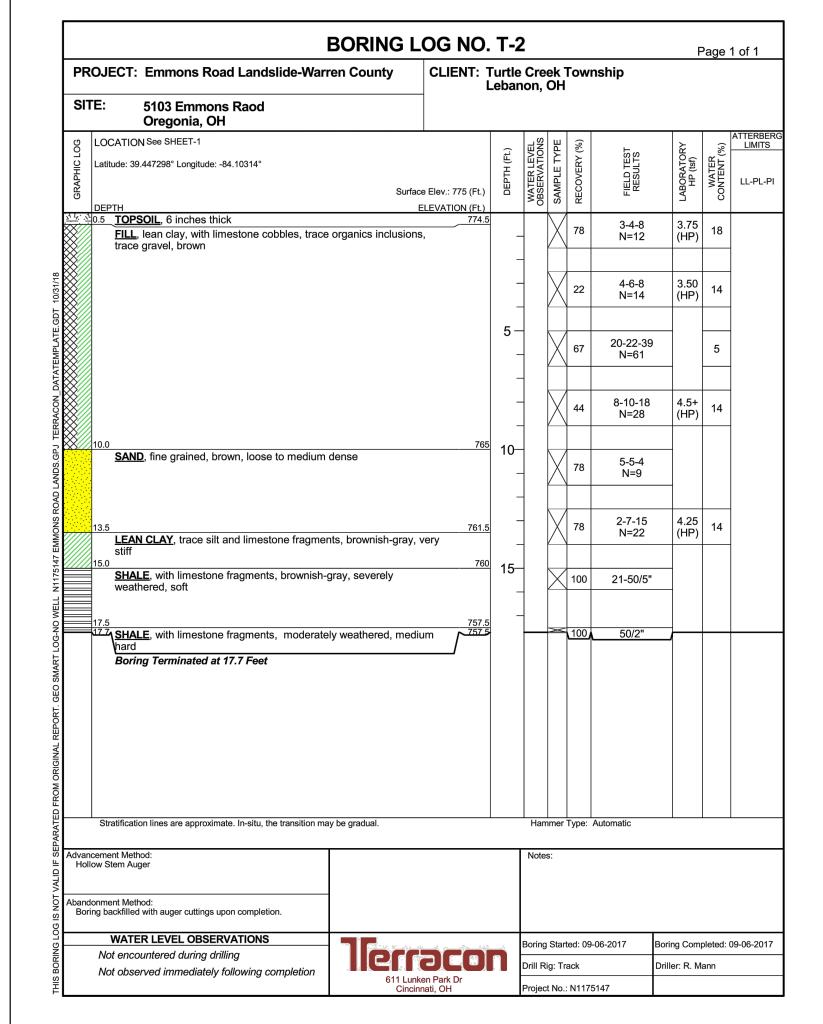
DAVID W. 💃 WESTENDORF 💃 🥦 74346

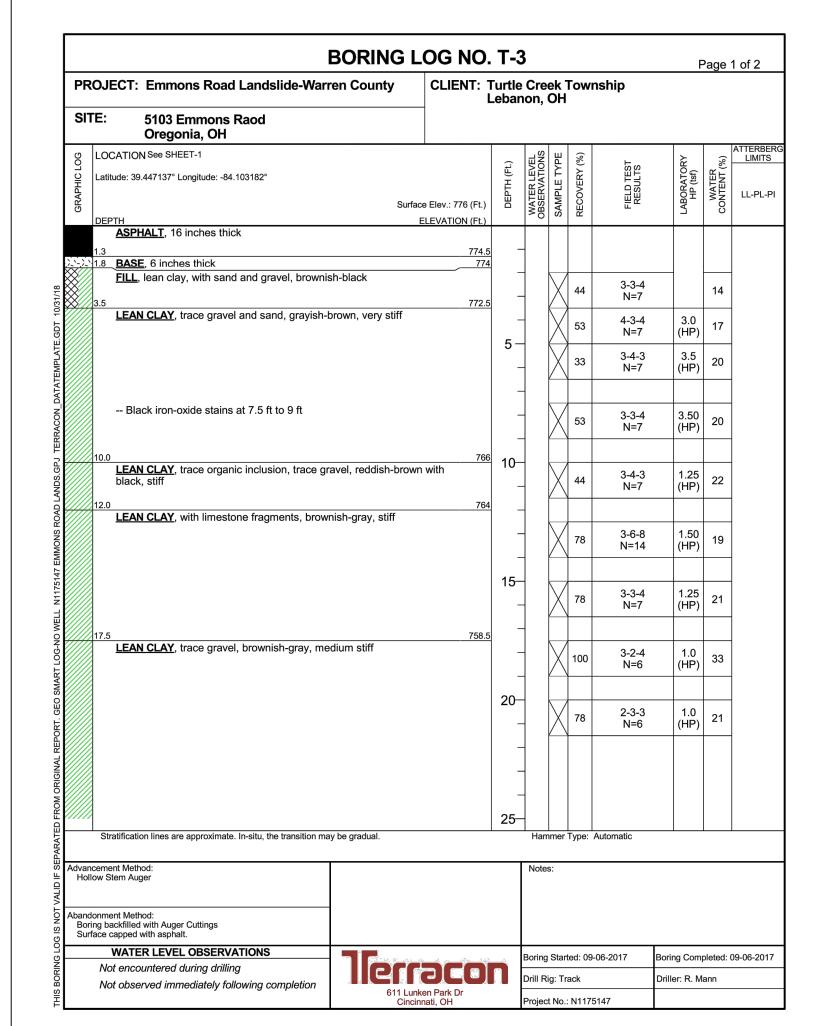
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DRAWN BY:	KM			
APPVD. BY:	DWW			
SCALE:	AS SHOWN			
DATE:	02/26/2019			
JOB NO.	N1185468			
ACAD NO.	RW.DWG			
SHEET NO.:	2			

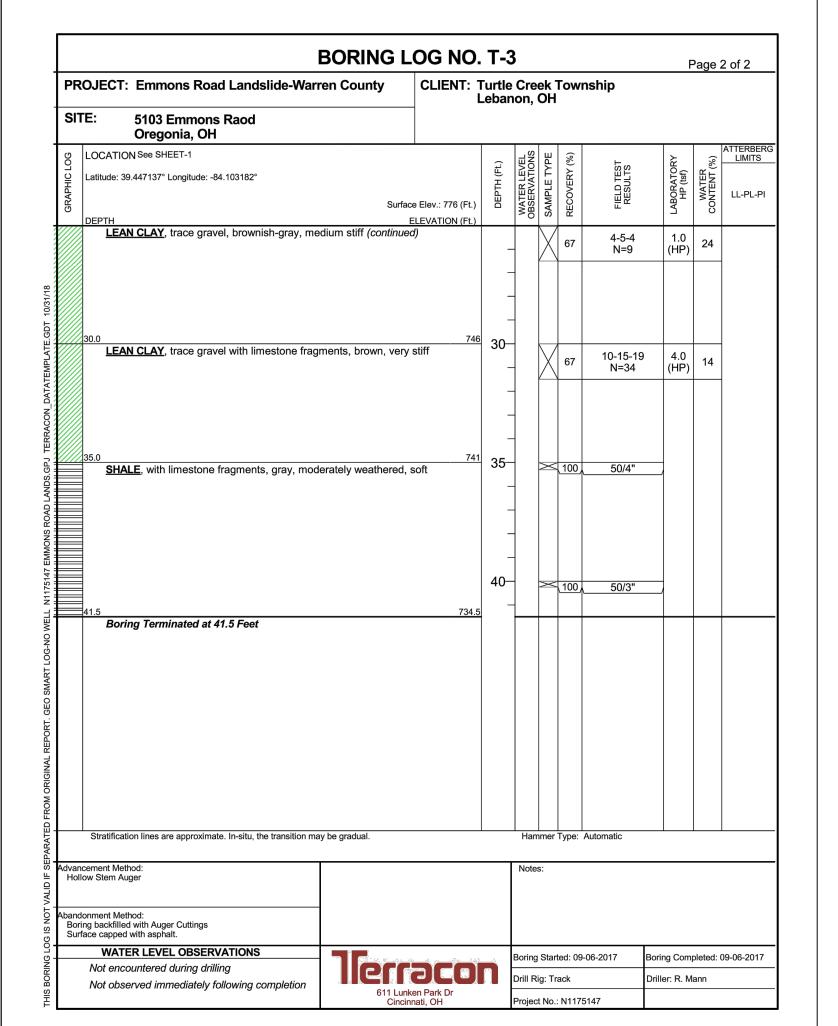
















°DAVID W. \* WESTENDORF \* \*

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SHEET 3

AS SHOWN 02/26/2019 N1185468

RW.DWG

DRAWN BY: KM

JOB NO.

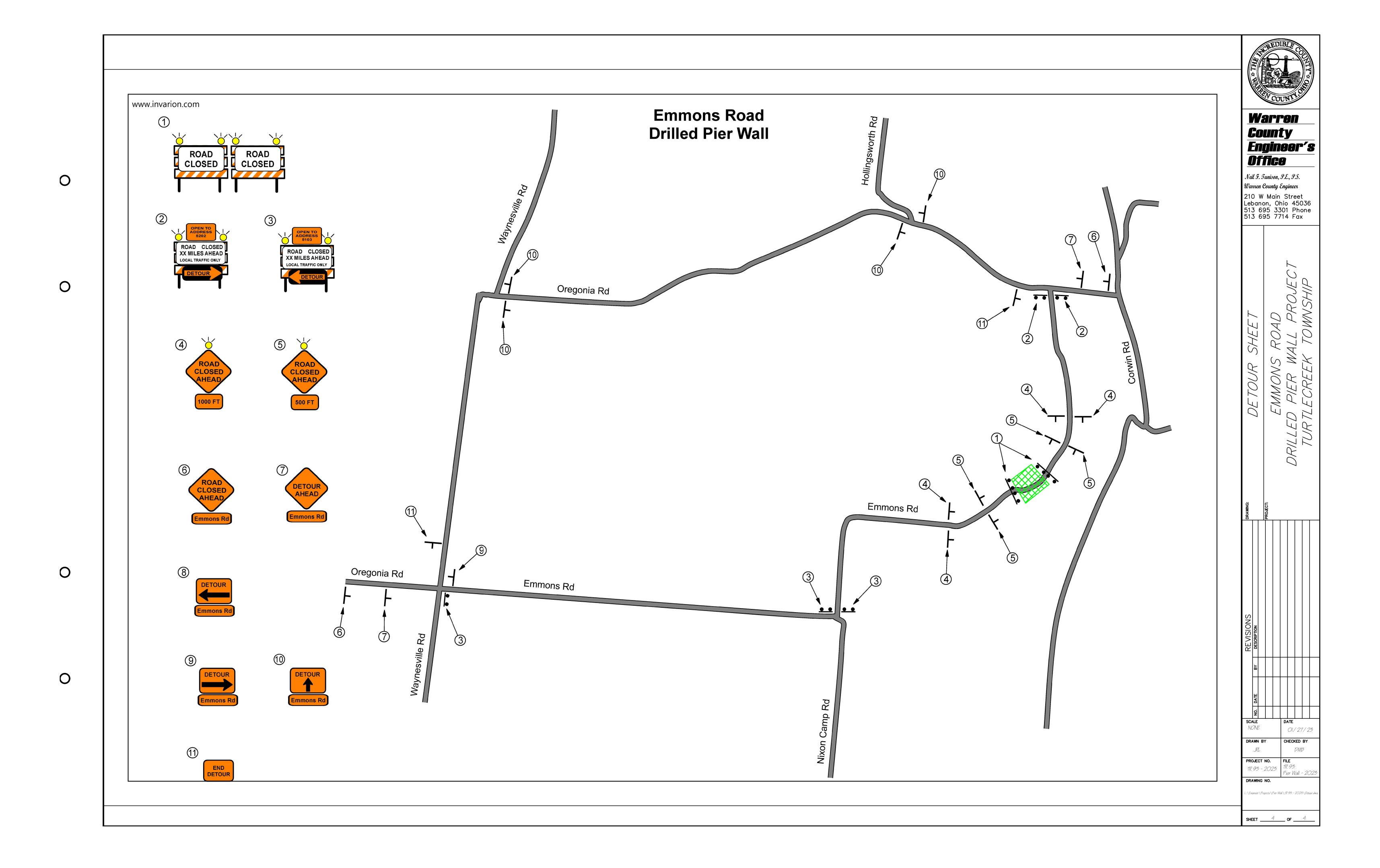
ACAD NO.

SHEET NO.:

APPVD. BY: DWW

OAD LANDSLIDE - WARREN COUNTY

TLECREEK TOWNSHIP
5103 EMMONS ROAD
ONIA, WARREN COUNTY, OHIO TOGS

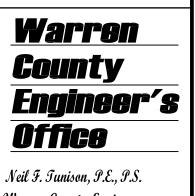


# DRILLED PIER WALL PROJECT LIBERTY KEUTER ROAD

### TURTLECREEK TOWNSHIP WARREN COUNTY, OHIO

VICINITY MAP NOT TO SCALE

OLD 122	TOWNSHIP LINE RI	RED OAK RD
DRAKE RO	SHOEMAKER RD	ON THE PROPERTY OF THE PROPERT
Lebanon	PROJEC	HOLLINGSWORTH RD
MONROE RD  MONROE RD  MOOKINGBIS AVE  48	EGONIA RD	EMMONS RD
STUBBS MILLS	SRY-KEUTER RD LIBERTY-KEUTER RD WAYNESWILE RD	NIXON



Lebanon, Ohio 45036 513 695 3301 Phone 513 695 7714 Fax

DATE: \_\_\_\_\_ WARREN COUNTY COMMISSIONER

DATE: \_\_\_\_ WARREN COUNTY COMMISSIONER

DATE: \_\_\_\_\_ WARREN COUNTY COMMISSIONER

WARREN COUNTY ENGINEER

APPROVED:

APPROVED: \_\_\_\_\_

APPROVED: \_\_\_\_\_

PLANS PREPARED BY: THE WARREN COUNTY ENGINEERING DEPARTMENT 210 W MAIN STREET LEBANON, OHIO 45036

SHEET NUMBER

TITLE SHEET PIER WALL PLAN & PROFILE SHEET DETAIL SHEET BORING LOGS SHEET DETOUR SHEET

UNDERGROUND UTILITIES

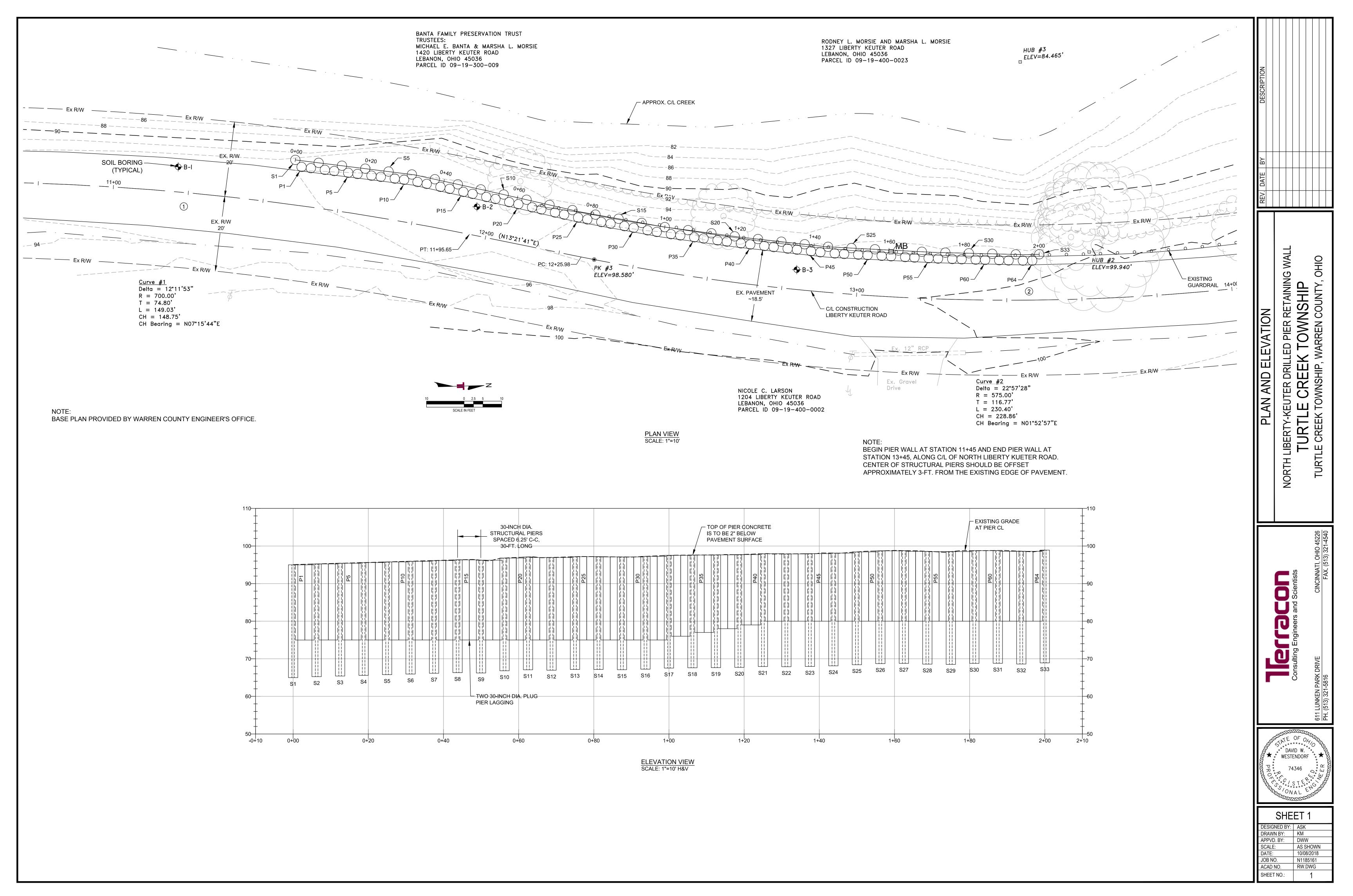
2 WORKING DAYS BEFORE YOU DIG

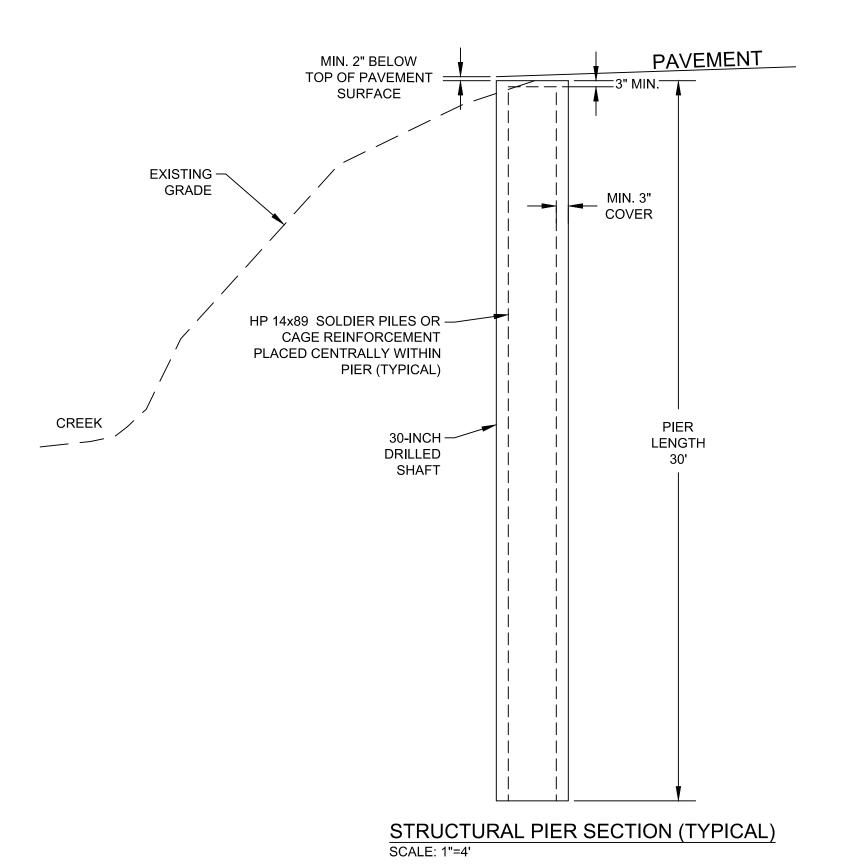
CALL TOLL FREE 800-362-2764

OHIO UTILITIES PROTECTION SERVICE

NON-MEMBERS MUST BE

CALLED DIRECTLY





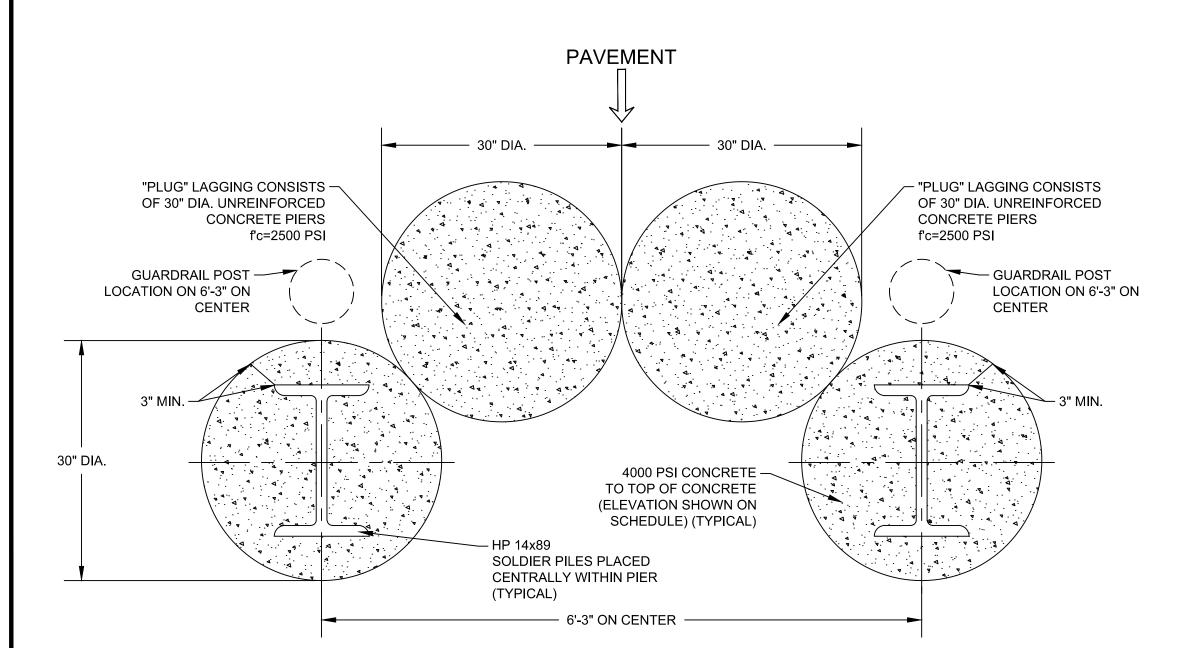
STROCTORAL FIER SCHEDOLL						
		Approximate Top of Pier	Diameter	Approximate Bottom of	Length of	
Pier No:	Station	Reference Elevation (ft.)	(inches)	Pier Reference	Pier (ft.)	
			(	Elevation (ft.)		
S1	0+00	94.8	30	64.8	30	
S2	0+6.25	95.0	30	65.0	30	
S3	0+12.5	95.2	30	65.2	30	
S4	0+18.75	95.4	30	65.4	30	
<b>S</b> 5	0+25	95.6	30	65.6	30	
S6	0+31.25	95.7	30	65.7	30	
<b>S</b> 7	0+37.5	95.9	30	65.9	30	
S8	0+43.75	96.1	30	66.1	30	
S9	0+50	96.0	30	66.0	30	
S10	0+56.25	96.6	30	66.6	30	
S11	0+62.5	96.9	30	66.9	30	
S12	0+68.75	96.8	30	66.8	30	
S13	0+75	97.0	30	67.0	30	
S14	0+81.25	97.0	30	67.0	30	
S15	0+87.5	96.9	30	66.9	30	
S16	0+93.75	97.0	30	67.0	30	
S17	1+00	97.3	30	67.3	30	
S18	1+6.25	97.4	30	67.4	30	
S19	1+12.5	97.4	30	67.4	30	
S20	1+18.75	97.5	30	67.5	30	
S21	1+25	97.7	30	67.7	30	
S22	1+31.25	97.7	30	67.7	30	
S23	1+37.5	97.7	30	67.7	30	
S24	1+43.75	97.9	30	67.9	30	
S25	1+50	98.2	30	68.2	30	
S26	1+56.25	98.5	30	68.5	30	
S27	1+62.5	98.6	30	68.6	30	
S28	1+68.75	98.4	30	68.4	30	
S29	1+75	98.3	30	68.3	30	
S30	1+81.25	98.6	30	68.6	30	
S31	1+87.5	98.6	30	68.6	30	
S32	1+93.75	98.4	30	68.4	30	
S33	2+00	98.7	30	68.7	30	

STRUCTURAL PIER SCHEDULE

Pier No:	Reference Elevation (ft.)	(inches)	Reference Elevation (ft.)	Pier (ft.)
			• •	
P1	94.8	30	75.0	19.8
P2	95.0	30	75.0	20.0
P3	95.0	30	75.0	20.0
P4	95.2	30	75.0	20.2
P5	95.2	30	75.0	20.2
P6	95.4	30	75.0	20.4
P7	95.4	30	75.0	20.4
P8 P9	95.6 95.6	30 30	75.0 75.0	20.6 20.6
P10	95.7	30	75.0	20.7
P10	95.7	30	75.0	20.7
P12	95.9	30	75.0	20.7
P13	95.9	30	75.0	20.9
P14	96.1	30	75.0	21.1
P15	96.1	30	75.0	21.1
P16	96.0	30	75.0	21.0
P17	96.0	30	75.0	21.0
P18	96.6	30	75.0	21.6
P19	96.6	30	75.0	21.6
P20	96.9	30	75.0	21.9
P21	96.9	30	75.0	21.9
P22	96.8	30	75.0	21.8
P23	96.8	30	75.0	21.8
P24	97.0	30	75.0	22.0
P25	97.0	30	75.0	22.0
P26	97.0	30	75.0	22.0
P27	97.0	30	75.0	22.0
P28	96.9	30	75.0	21.9
P29	96.9	30	75.0	21.9
P30	97.0	30	75.0	22.0
P31	97.0	30	75.0	22.0
P32	97.3	30	75.0	22.3
P33	97.3	30	76.0	21.3
P34	97.4	30	76.0	21.4
P35	97.4	30	77.0	20.4
P36	97.4	30	77.0	20.4
P37	97.4	30	78.0	19.4
P38	97.5	30	78.0	19.5
P39	97.5	30	79.0	18.5
P40	97.7	30	79.0	18.7
P41 P42	97.7	30	80.0	17.7
P43	97.7 97.7	30 30	80.0 80.0	17.7 17.7
P44	97.7	30	80.0	17.7
P45	97.7	30	80.0	17.7
P46	97.9	30	80.0	17.9
P47	97.9	30	80.0	17.9
P48	98.2	30	80.0	18.2
P49	98.2	30	80.0	18.2
P50	98.5	30	80.0	18.5
P51	98.5	30	80.0	18.5
P52	98.6	30	80.0	18.6
P53	98.6	30	80.0	18.6
P54	98.4	30	80.0	18.4
P55	98.4	30	80.0	18.4
P56	98.3	30	80.0	18.3
P57	98.3	30	80.0	18.3
P58	98.6	30	80.0	18.6
P59	98.6	30	80.0	18.6
P60	98.6	30	80.0	18.6
P61	98.6	30	80.0	18.6
P62	98.4	30	80.0	18.4
P63	98.4	30	80.0	18.4
P64	98.7	30	80.0	18.7

PLUG PIER SCHEDULE

Approximate Top of Pier | Diameter | Approximate Bottom of Pier | Length of



ALTERNATE 1: STRUCTURAL PIER WITH HP REINFORCEMENT DETAIL AND "PLUG" PIER LAGGING SCALE: 1"=1'

	PAVEMENT
"PLUG" LAGGING CONSISTS — OF 30" DIA. UNREINFORCED CONCRETE PIERS f'c=2500 PSI	30" DIA. "PLUG" LAGGING CONSISTS OF 30" DIA. UNREINFORCED CONCRETE PIERS f'c=2500 PSI
GUARDRAIL POST – LOCATION ON 6'-3" ON CENTER	GUARDRAIL POST LOCATION ON 6'-3" ON CENTER
SIX #8 BARS FULL LENGTH— (BOTTOM OF PIER TO TOP, LESS 3")	SIX #8 BARS FULL LENGTH (BOTTOM OF PIER TO TOP, LESS 3")
3" MIN.	3" MIN.
30" DIA. 19"	4000 PSI CONCRETE TO TOP OF CONCRETE (ELEVATION SHOWN ON SCHEDULE) (TYPICAL)
#4 CLOSED STI 7" C-C FROM PI BASE TO TOP C LESS 3" (TYPICA	FR FPIER, L) TWO #8 BARS
BOTTOM OF PIER TO TOP, LESS 3"	— 6'-3" ON CENTER ————————————————————————————————————

ALTERNATE 2: STRUCTURAL PIER WITH CAGE REINFORCEMENT DETAIL AND "PLUG" PIER LAGGING

#### **DRILLED PIER CONSTRUCTION NOTES**

#### **DRILLED PIER INSTALLATION**

1. CONSTRUCT THE 30-INCH DIAMETER DRILLED STRUCTURAL PIER WITH PLUG PIER LAGGING RETAINING WALL USING EITHER ROLLED STEEL SECTIONS OR CAGE REINFORCEMENT AS SHOWN AND DESCRIBED ON PLANS. THE PURPOSE OF THIS WORK IS TO CONTROL LATERAL CREEP-TYPE MOVEMENT ON THE DOWNSLOPE (WEST) SIDE OF LIBERTY-KEUTER ROAD. CONTRACTOR IS RESPONSIBLE FOR CLEARING OVERHEAD AND UNDERGROUND UTILITIES AND PROVIDING ACCESS FOR EQUIPMENT. THE PIER EXCAVATION SHALL BE UNCLASSIFIED. TURTLECREEK TOWNSHIP

WILL CLOSE THIS SECTION OF LIBERTY-KEUTER ROAD TO TRAFFIC DURING PIER WALL CONSTRUCTION.

- 2. THE REINFORCED DRILLED PIER RETAINING WALL WILL CONSIST OF DRILLED PIERS SPACED APPROXIMATELY ON 6 FEET -3 INCH (6'-3") ON CENTER, AS SHOWN ON THE SITE PLAN DRAWING. APPROXIMATE BOTTOM OF DRILLED PIERS HAS BEEN SHOWN ON PIER SCHEDULE TABLE ON THIS SHEET; HOWEVER, ACTUAL LENGTHS WILL BE BASED ON ACTUAL FIELD CONDITIONS AS DETERMINED BY THE GEOTECHNICAL CONSULTANT.
- 3. THE PIERS SHALL BE LOCATED AS SHOWN ON PLAN WITHIN 6" OF PLAN LOCATION. THE PIERS SHALL MAINTAIN A PLUMBNESS DEVIATION OF A MAXIMUM OF 1" IN 12 FT. VERTICAL HEIGHT. A MINIMUM 3" CONCRETE COVER BETWEEN THE REINFORCING STEEL AND THE EXTERIOR (SIDES AND TOP) OF THE DRILLED PIER SHALL BE PROVIDED.
- 4. TWO ALTERNATIVES FOR REINFORCEMENT OF STRUCTURAL PIERS HAVE BEEN PROVIDED. REINFORCEMENT FOR THE STRUCTURAL PIERS SHALL CONSIST EITHER OF ROLLED STEEL SECTIONS HAVING A YIELD STRENGTH OF 50 KSI OR CAGE REINFORCING STEEL HAVING A YIELD STRENGTH OF 60 KSI. ROLLED STEEL SECTIONS SHOULD CONSIST OF HP 14X89 BEAMS FOR ALTERNATIVE 1 AS NOTED ON THESE PLANS, THE CONFIGURATION OF THE REINFORCING CAGE FOR ALTERNATIVE 2 HAS BEEN SHOWN ON SHEET 2 AND CONSISTS OF A 14"X19" RECTANGULAR CAGE WITH A LAYER
- 5. THE PRE-DRILLED SHAFT WILL BE BACKFILLED WITH CONCRETE TO THE TOP OF CONCRETE ELEVATION, USING FREE FALL METHODS, UNLESS WATER CANNOT BE REMOVED FROM THE PIER HOLE. IF THERE IS MORE THAN 2-INCHES OF WATER IN THE PIER AT THE TIME OF CONCRETE PLACEMENT, PLEASE SEE ITEM 8 BELOW. THE TOP OF PIER CONCRETE ELEVATION SHALL BE ABOUT 2 INCHES BELOW THE TOP OF PAVEMENT ELEVATION, TO AVOID CONTACT BY SNOW PLOWS. STRUCTURAL PIER CONCRETE SHALL BE CONCRETE (F'C = 4000 PSI, MAXIMUM SLUMP = 4 INCHES, A/E = 6± 2%) PLACED USING FREE FALL METHOD OF PLACEMENT. CONCRETE SHALL BE PLACED INTO EACH PIER EXCAVATION ON THE SAME DAY THAT THE DRILLING IS COMPLETED.
- 6. TEMPORARY STEEL CASING SHOULD BE ON-SITE AND USED WHEREVER REQUIRED TO STABILIZE LOOSE OR CAVING MATERIALS, OR TO SEAL OFF WATER BEARING ZONES ENCOUNTERED DURING CONSTRUCTION.
- 7. THE CONTRACTOR SHALL MAINTAIN A RECORD OF EACH PIER DRILLED, WHICH WILL INCLUDE AS A MINIMUM: PIER NUMBER; GROUND ELEVATION; DEPTH OR ELEVATION WHERE GROUNDWATER WAS ENCOUNTERED; PIER TOP ELEVATION; ELEVATION OF BOTTOM OF PIER OR PIER LENGTH; ELEVATION OF THE TOP OF THE PIER CONCRETE; DATE DRILLED; DATE COMPLETED; AND WEATHER CONDITIONS.
- 8. IT IS ANTICIPATED THAT WATER MAY ENTER SOME OF THE PIER EXCAVATIONS. THE DEPTH OF PONDED WATER AT THE BOTTOM OF THE PIER EXCAVATIONS SHOULD NOT EXCEED 2 INCHES, PRIOR TO PLACING CONCRETE. IF THE WATER CANNOT BE PUMPED DOWN, TREMIE PLACEMENT METHODS WILL BE REQUIRED.
- 9. THE DRILLED PIER EXCAVATIONS SHOULD BE INSPECTED BY A QUALIFIED GEOTECHNICAL REPRESENTATIVE TO CONFIRM THAT THE DRILLED PIERS HAVE BEEN EXTENDED TO THE DESIGN PIER LENGTH, AND THAT THE DRILLED PIERS HAVE BEEN CONSTRUCTED PER SPECIFICATIONS.
- 10.PIER SPOILS SHALL BE TRUCKED FROM THE SITE (NOT WASTED ON THE HILLSIDE). NO FILL PLACEMENT SHOULD BE ALLOWED DOWNSLOPE OF THE WALL.

#### PLUG PIER INSTALLATION

- 1. TWO 30-INCH DIAMETER PLUG PIERS CONSISTING OF UNREINFORCED CONCRETE (F'C = 2500 PSI, MAXIMUM SLUMP = 4 INCHES), AS NOTED ON THE CROSS-SECTION DETAILS. THE PLUG PIERS WILL ACT AS LAGGING FOR THE PIER WALL.
- 2. PLUG PIER INSTALLATION SHALL BEGIN AFTER THE STRUCTURAL PIER ELEMENTS HAVE GAINED STRENGTH (AT LEAST 24 HOURS AFTER PLACEMENT OF STRUCTURAL PIER CONCRETE).
- 3. THE TOP OF THE PLUG PIER CONCRETE SHALL BE ABOUT 2 INCHES BELOW THE PAVEMENT.
- 4. THE BOTTOM OF ALL THE PLUG PIERS INSTALLED SHOULD EXTEND TO THE ELEVATIONS DETAILED IN THE DRILLED
- PIER SCHEDULE.
- 5. PLUG PIER SPOILS SHALL BE TRUCKED FROM THE SITE (NOT WASTED ON THE HILLSIDE).

#### DRAINAGE AND OTHER CONSTRUCTION CONSIDERATIONS

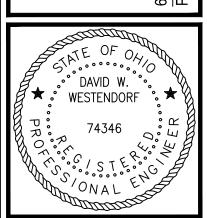
- 1. NEW STRUCTURAL FILL MAY BE PLACED ON UPSLOPE SIDE OF PIERS TO RETAIN GRADE NEXT TO THE EDGE OF PAVEMENT. SONOTUBES OR EQUIVALENT WILL BE REQUIRED IN SOME AREAS, DUE TO THE STEEPLY SLOPING HILLSIDE BELOW THE WALL. FILL SHOULD BE PLACED AND COMPACTED PER ODOT SPECIFICATIONS (ITEM 203). ONLY HAND-OPERATED EQUIPMENT SHOULD BE USED WITHIN 5 FEET OF THE FRONT OF THE PIERS.
- 2. EXISTING GUARDRAIL WILL BE REMOVED AND REPLACED IN THIS PROJECT.

#### FIELD QUALITY CONTROL

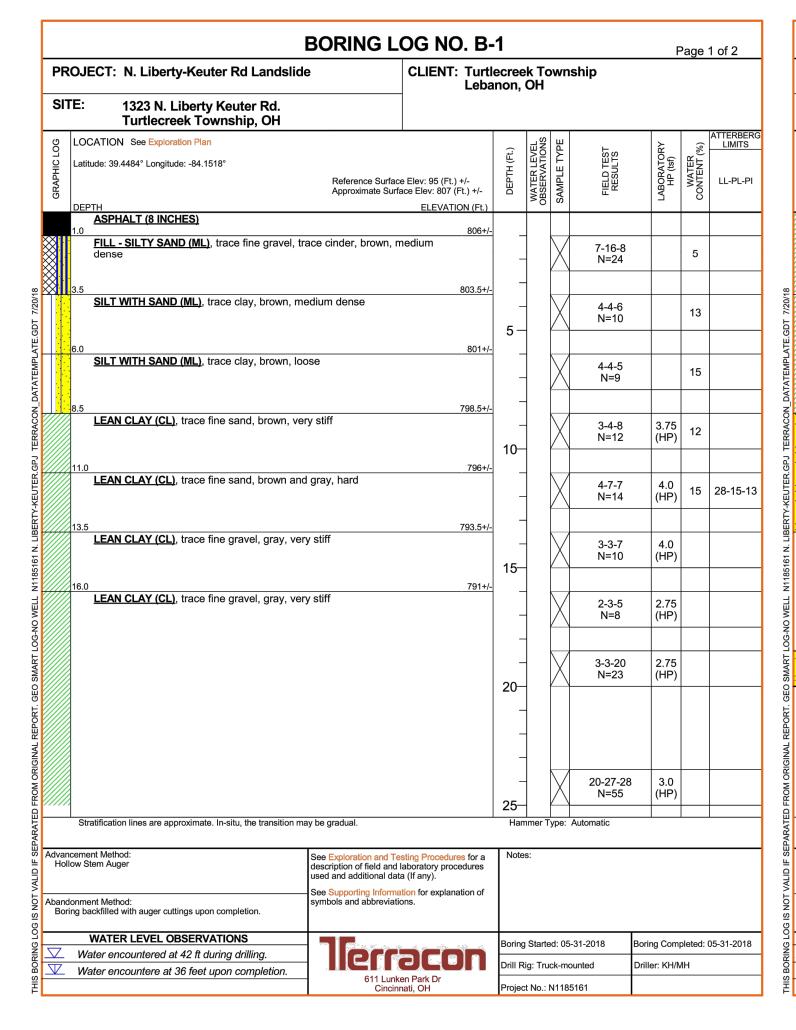
- A. OWNER WILL COORDINATE FIELD CONSTRUCTION INSPECTION AND REPORTING THROUGH IN-HOUSE PERSONNEL OR TERRACON. DOCUMENTATION SHALL INCLUDE THE FOLLOWING AT EACH PIER:
- GROUND ELEVATION 2. AS-BUILT PIER DIAMETER AND TOP AND BOTTOM PIER ELEVATIONS.
- 3. AS-BUILT PIER LENGTH.
- 4. DESCRIPTION OF ENCOUNTERED SOIL MATERIALS.
- 5. DESCRIPTION, LOCATION, AND DIMENSIONS OF OBSTRUCTIONS.
- FINAL TOP CENTERLINE LOCATION AND DEVIATIONS FROM REQUIREMENTS.
- 7. VARIATION OF SHAFT FROM PLUMB.
- DRILLED SHAFT EXCAVATING METHOD. 9. LEVELNESS OF PIER BOTTOM AND ADEQUACY OF CLEANOUT.
- 10. GROUND-WATER CONDITIONS AND WATER-INFILTRATION RATE, DEPTH, AND PUMPING.
- 11.DESCRIPTION, DIAMETER, AND TOP AND BOTTOM ELEVATIONS OF TEMPORARY OR 12.DESCRIPTION OF SOIL OR WATER MOVEMENT, SIDEWALL STABILITY, LOSS OF GROUND, AND MEANS OF CONTROL.
- 13. DATE AND TIME OF STARTING AND COMPLETING DRILLED PIER EXCAVATION.
- 14. POSITION OF REINFORCING STEEL.
- 15. CONCRETE PLACEMENT METHOD, INCLUDING DELAYS. 16. ELEVATION OF CONCRETE DURING REMOVAL OF CASINGS.
- 17.LOCATIONS OF CONSTRUCTION JOINTS, IF ANY.
- 18. REMARKS, UNUSUAL CONDITIONS ENCOUNTERED, AND DEVIATIONS FROM REQUIREMENTS.

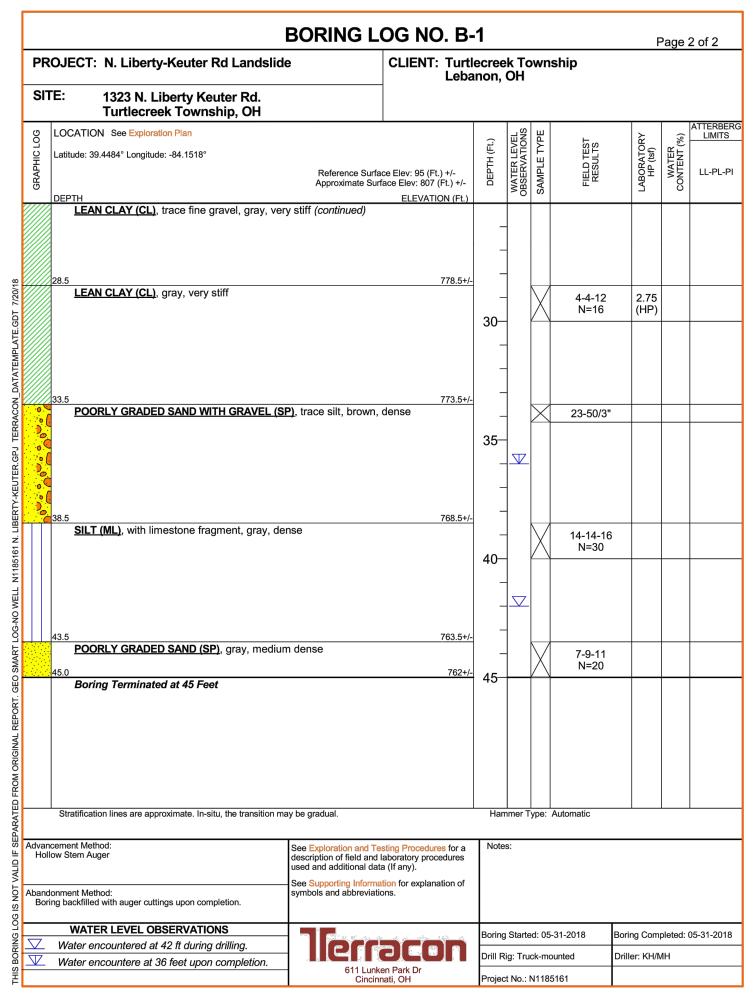
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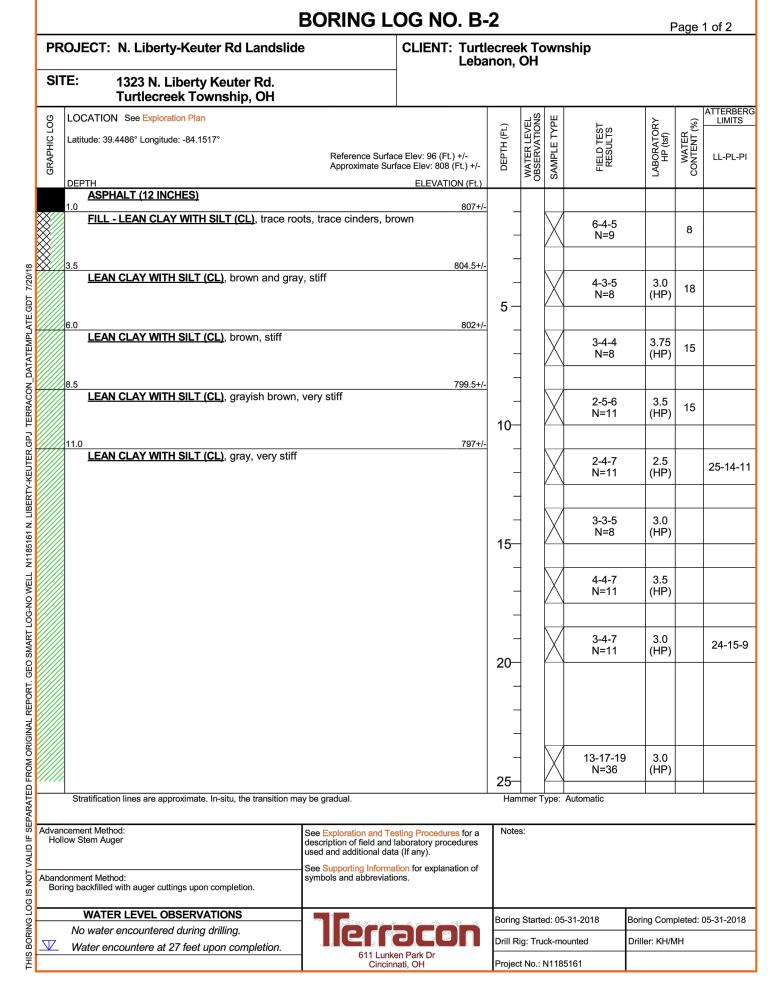
AILS



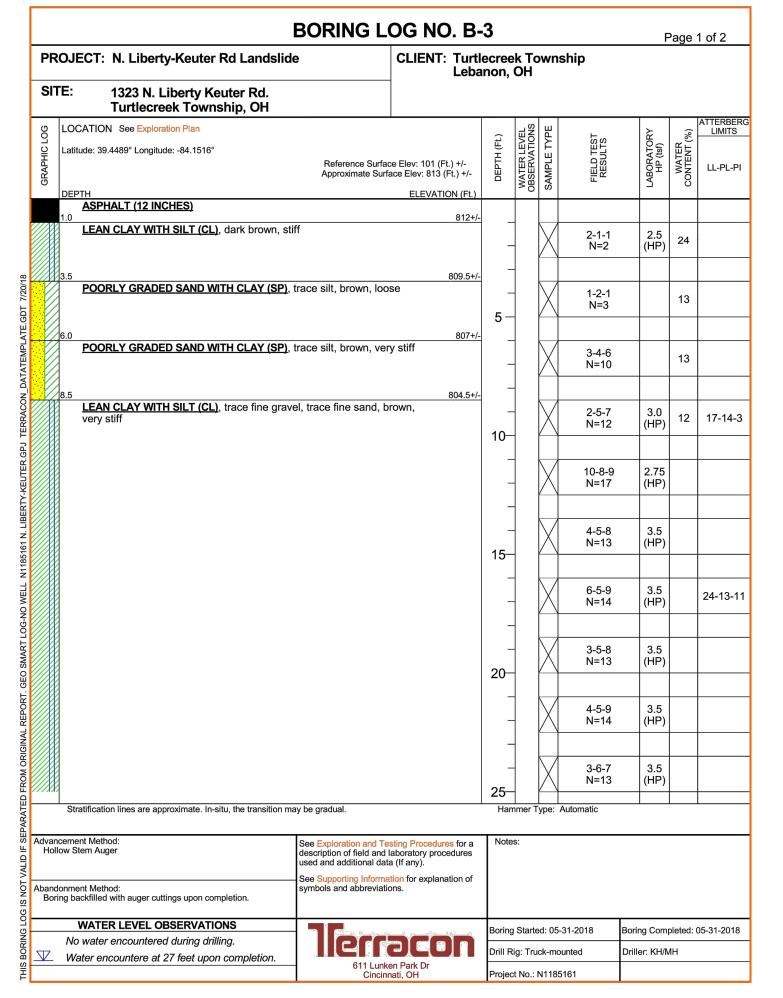
SHEET 2			
DESIGNED BY:	ASK		
DRAWN BY:	KM		
APPVD. BY:	DWW		
SCALE:	AS SHOWN		
DATE:	10/08/2018		
JOB NO.	N1185161		
ACAD NO.	RW.DWG		
SHEET NO.:	2		

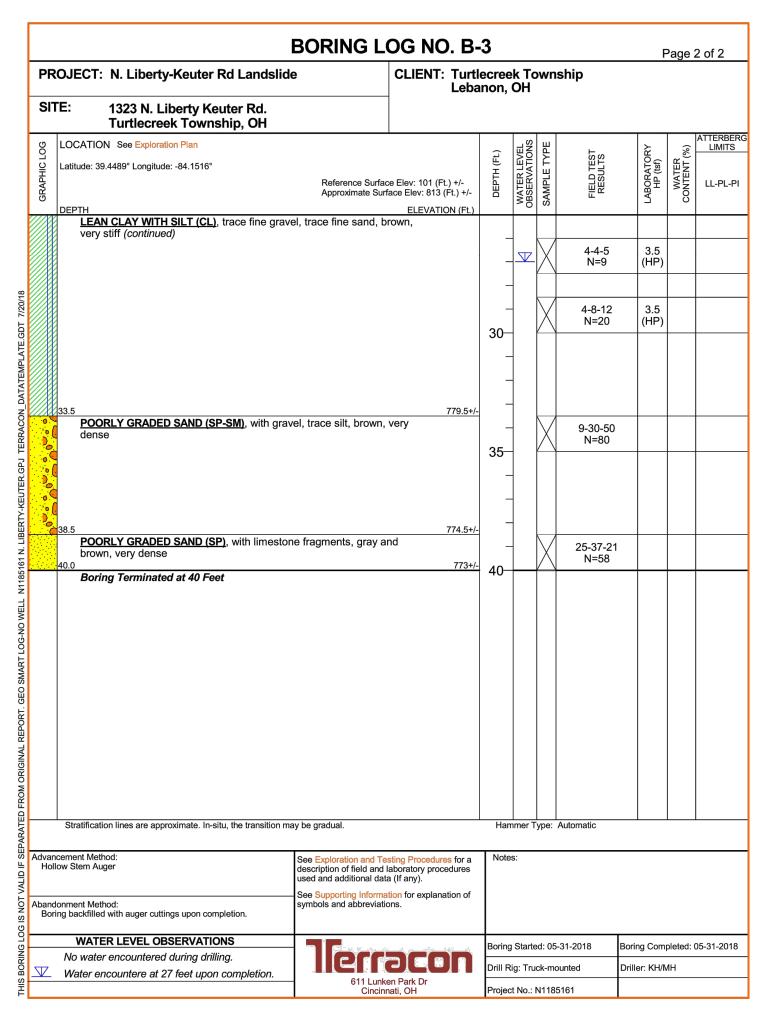






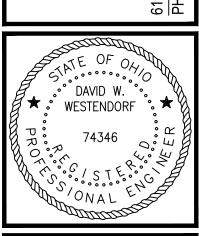
E: 1323 N. Liberty Keuter Rd.	CLIENT: Turtlecreek Township Lebanon, OH
Turtlecreek Township, OH  LOCATION See Exploration Plan  Latitude: 39.4486° Longitude: -84.1517°  DEPTH	Reference Snutace Elen: 808 (Lt.) +/- Abblookarto RY HP (ss)  CONTENT  MATER LEVEL  WATER LEVEL  SAMPLE TYPE  SAMPLE TYPE  CONTENT(%)  CONTENT  RESULTS  RESULTS  SAMPLE TYPE  CONTENT  WATER LEVEL  BETEATON (Etr.)  LABORATOR  WATER  CONTENT  WATER  SAMPLE TYPE  CONTENT  WATER  SAMPLE TYPE  RESULTS  R
33.5  POORLY GRADED SAND (SP), trace gravel, tra 35.0  Boring Terminated at 35 Feet  Stratification lines are approximate. In-situ, the transition may be	774.5+/- 2 silt, brown, very dense 773+/- 35  774.5+/- 773+/- 35
	Exploration and Testing Procedures for a ription of field and laboratory procedures



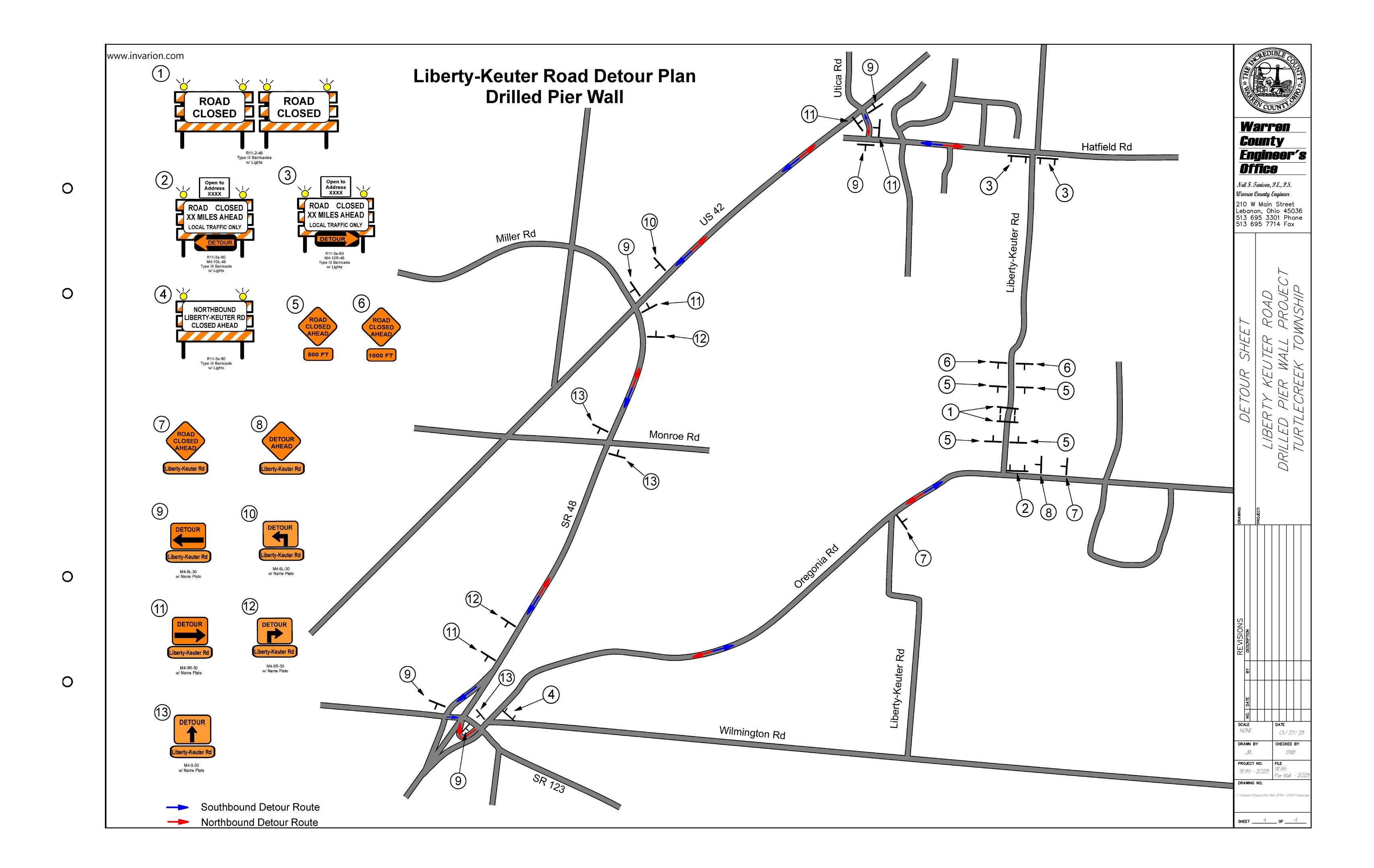




S and Scientists	CINCINNATI, OHIO 452 FAX. (513) 321-45
TEFFEE Consulting Engineers and Scientists	EN PARK DRIVE 321-5816



AL EXAM
ET 3
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AS SHOWN
10/08/2018
N1185161
RW.DWG
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## DRILLED PIER WALL PROJECT OLIVE BRANCH ROAD

WASHINGTON TOWNSHIP WARREN COUNTY, OHIO

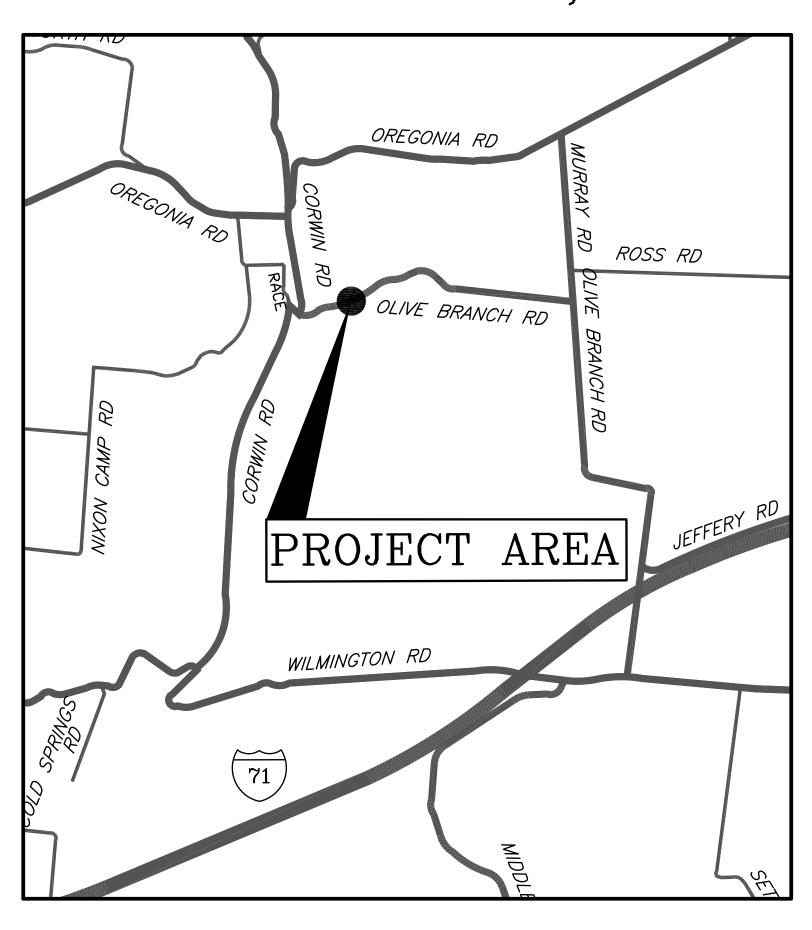
### CONVENTIONAL SIGNS

PK NAIL (SET) HUB & TACK (SET) EX. POLE EX. TREELINE EX. & PROP. GUARDRAIL EX. AERIAL ELECTRIC EX. GUY WIRE EX. FLOW LINE EX. FIBER LINE

### SHEET NUMBER

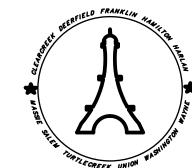
2-4

TITLE SHEET PIER WALL PLAN SHEET DETOUR SHEET



VICINITY MAP NOT TO SCALE

APPROVED:			
DATE:			
APPROVED:			
DATE:	WARREN	COUNTY	COMMISSIONER
ADDDOLUD			
APPROVED:			
DATE:	WARREN	COUNTY	COMMISSIONER
4 D D D 0 1 1 1 D			
APPROVED:			
DATE:	WARREN	COUNTY	COMMISSIONER



#### PLANS PREPARED BY:

THE WARREN COUNTY ENGINEERING DEPARTMENT 210 W. MAIN STREET LEBANON, OHIO 45036

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T.	PURTLECREEK UNION WASH	g.

NGINEER\PROJECTS\Pier Walls\CR217-2023\

SHEET  $\underline{\hspace{1cm}}$  OF  $\underline{\hspace{1cm}}$  5

**Warren** 

**Engineer's** 

Neil F. Tunison, P.E., P.S. Warren County Engineer

210 W. Main Street Lebanon, Ohio 45036 513 695 3301 Phone 513 695 7714 Fax

**County** 

Office

UNDERGROUND UTILITIES 2 WORKING DAYS BEFORE YOU DIG CALL TOLL FREE 800-362-2764 OHIO UTILITIES PROTECTION SERVICE

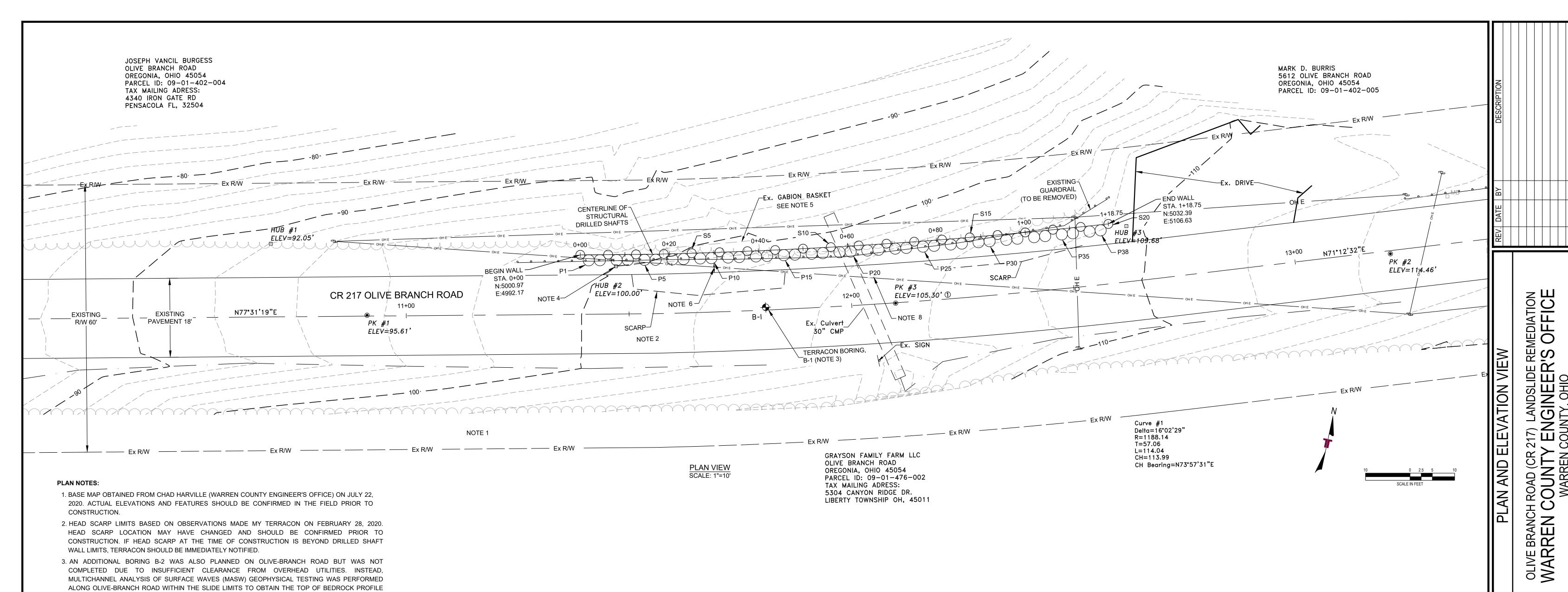
> NON-MEMBERS MUST BE CALLED DIRECTLY

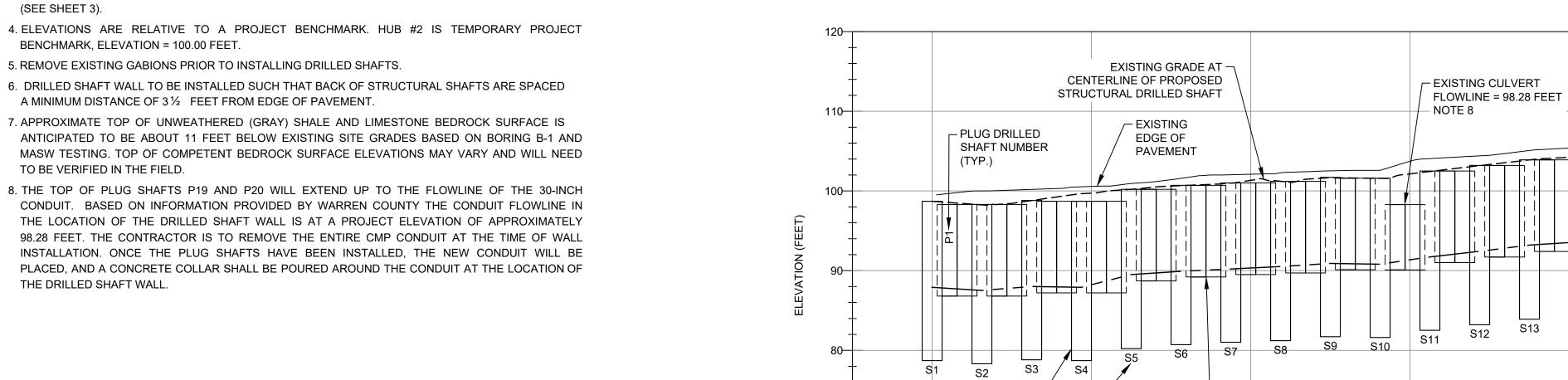
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24-INCH DIAMETER -

SHAFT (TYP.)

— STRUCTURAL DRILLED -

STRUCTURAL DRILLED — SHAFT NUMBER (TYP.)

0+00

-0+10

S19 S18 S17 S15 S16 S13 S14 S12  $\vdash$  ESTIMATED TOP OF UNWEATHERED SHALE AND LIMESTONE BEDROCK (2) 30-INCH DIAMETER UNREINFORCED PLUG \_ SHAFTS (TYP.) 1+20 STATIONS ALONG DRILLED SHAFT RETAINING WALL ALIGNMENT

- TOP OF DRILLED SHAFT

EDGE OF PAVEMENT

CONCRETE IS TO BE 3" BELOW EXISTING GRADE AT EXISTING

ELEVATION VIEW SCALE: 1"=10' H&V

JEFFERY D.

DUNLAP

E-63301

W

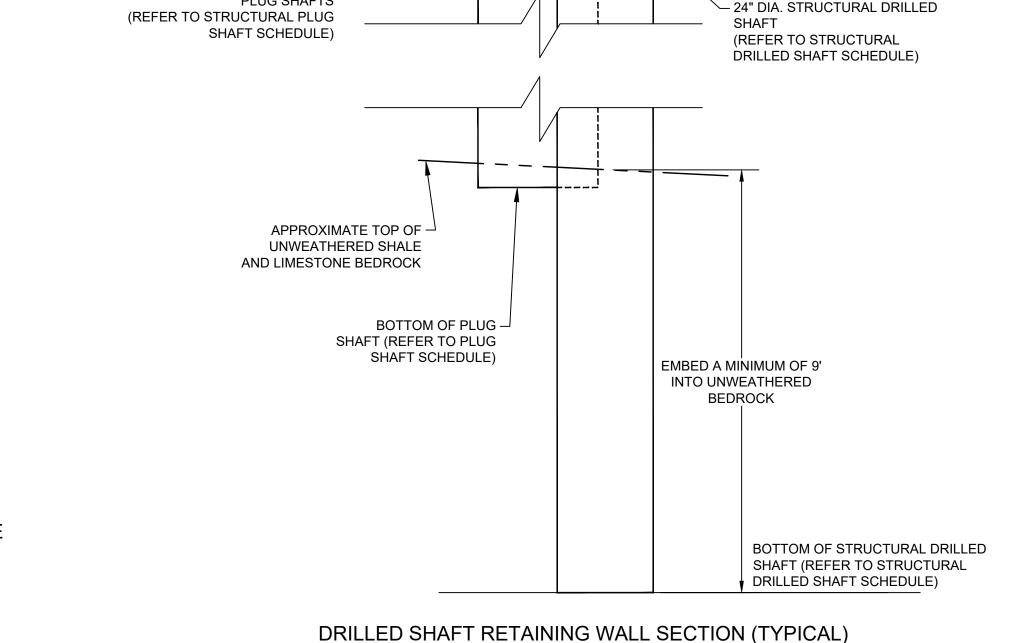
ON A L

DUNCH

DOWN

D

40					
SHEET 2					
DESIGNED BY:	ASK/DWW				
DRAWN BY:	KM				
APPVD. BY:	JDD				
SCALE:	AS SHOWN				
DATE:	07/29/2020				
JOB NO.	N1205222				
ACAD NO.	WC RW.DWG				
SHEET NO.:	2				



MINIMUM DISTANCE FROM

- FORM EXPOSED

AS NEEDED

PORTION OF SHAFT

- EXISTING GABIONS **REMOVE PRIOR TO** 

— EXISTING SLOPE

**GRADE VARIES** 

DRILLED SHAFT

INSTALLATION

**BACK OF STRUCTURAL** 

DRILLED SHAFT TO EDGE

OF ROADWAY

GUARDRAIL

**POSTS AT 6'-3"** 

STRUCTURAL

DRILLED SHAFT

ADJACENT TO EACH

EXISTING PAVEMENT (BY OTHERS)

TWO 30" DIA. UNREINFORCED

PLUG SHAFTS

"PLUG" DRILLED SHAFT LAGGING -- "PLUG" DRILLED SHAFT LAGGING CONSISTS OF 30" DIA. UNREINFORCED CONSISTS OF 30" DIA. UNREINFORCED CONCRETE SHAFTS (f'c=2500 PSI, AE) CONCRETE SHAFTS (f'c=2500 PSI, AE) **EMBEDDED A MINIMUM OF 6-INCHES** EMBEDDED A MINIMUM OF 6-INCHES INTO UNWEATHERED SHALE AND INTO UNWEATHERED SHALE AND LIMESTONE BEDROCK LIMESTONE BEDROCK **GUARDRAIL POST** - GUARDRAIL POST LOCATION ON 6'-3" ON LOCATION ON 6'-3" ON CENTER CENTER FOUR #10 BARS - FOUR #10 BARS FULL LENGTH FULL LENGTH (BOTTOM OF SHAFT (BOTTOM OF SHAFT TO TOP, LESS 3") TO TOP, LESS 3") **UPSLOPE** STRUCTURAL DRILLED SHAFT f'c=4000 PSI, AE CONCRETE TO TOP OF CONCRETE #3 CLOSED STIRRUPS ELEVATION SHOWN ON GRADE 60 SCHEDULE (TYPICAL) 16" C-C FROM PIER BASE TO TOP OF SHAFT, LESS 3" TWO #10 BARS -- TWO #10 BARS (TYPICAL) **BOTTOM OF SHAFT BOTTOM OF SHAFT** TO TOP, LESS 3" TO TOP, LESS 3" 6'-3" ON CENTER

**PAVEMENT** 

(ALTERNATE 2): STRUCTURAL DRILLED SHAFT WITH STEEL CAGE REINFORCEMENT DETAIL AND "PLUG" SHAFT LAGGING

STRUCTURAL SHAFT SCHEDULE							
Shaft No.	Shaft No. Diameter		Approx. Top of Shaft Concrete Elevation <sup>(1)</sup>	Approx. Top of Unweathered Bedrock Elevation <sup>(2)</sup>	Approximate Drilled Shaft Bottom Elevation	Design Drilled Shaft Length <sup>(3)</sup>	
	Inches	Feet	Feet	Feet	Feet	Feet	
S1	24	0+00	98.7	87.9	78.7	20	
S2	24	0+06.25	98.3	87.5	78.3	20	
S3	24	0+12.5	98.8	88.0	78.8	20	
\$4	24	0+18.75	98.7	87.9	78.7	20	
S5	24	0+25	100.2	89.5	80.2	20	
S6	24	0+31.25	100.7	89.9	80.7	20	
<b>S</b> 7	24	0+37.5	101.0	90.2	81.0	20	
S8	24	0+43.75	101.2	90.5	81.2	20	
S9	24	0+50	101.7	90.9	81.7	20	
S10	24	0+56.25	101.6	90.8	81.6	20	
S11	24	0+62.5	102.5	91.7	82.5	20	
S12	24	0+68.75	103.2	92.4	83.2	20	
S13	24	0+75	103.9	93.2	83.9	20	
S14	24	0+81.25	104.3	93.6	84.3	20	
S15	24	0+87.5	105.1	94.3	85.1	20	
S16	24	0+93.75	105.3	94.6	85.3	20	
S17	24	1+00	105.9	95.2	85.9	20	
S18	24	1+6.25	106.5	95.7	86.5	20	
S19	24	1+12.25	107.1	96.3	87.1	20	
S20	24	1+18.75	108.1	97.4	88.1	20	

1) TOP OF SHAFT CONCRETE ELEVATION IS 3" BELOW THE APPROXIMATE GRADES AT THE EDGE OF PAVEMENT. 2) THE DESIGN HAS BEEN BASED ON A MAXIMUM DEPTH TO COMPETENT UNWEATHERED (GRAY) SHALE AND LIMESTONE BEDROCK OF 11-FEET BELOW EXISTING SITE GRADES. IF COMPETENT BEDROCK IS ENCOUNTERED AT A GREATER DEPTH DURING CONSTRUCTION, PLEASE NOTIFY TERRACON FOR FURTHER EVALUATION. 3) ACTUAL DRILLED SHAFT LENGTH CAN BE ALTERED IN FIELD BY CONTRACTOR, PROVIDED THE MINIMUM SOCKET LENGTH OF 9-FEET INTO UNWEATHERED BEDROCK IS ACHIEVED AND CONFIRMED BY GEOTECHNICAL PERSONNEL IN THE FIELD.

PLUG SHAFT SCHEDULE						
Plug Shaft No's.		Diameter Approx. 10p of Shaft Bottom			Approximate Drilled Shaft Length <sup>(2)</sup>	
		inches	feet	feet	feet	
P1	P2	30	98.3	86.8	11.5	
Р3	P4	30	98.3	86.8	11.5	
P5	P6	30	98.7	87.2	11.5	
P7	P8	30	98.7	87.2	11.5	
Р9	P10	30	100.2	88.7	11.5	
P11	P12	30	100.7	89.2	11.5	
P13	P14	30	101.0	89.5	11.5	
P15	P16	30	101.2	89.7	11.5	
P17	P18	30	101.6	90.1	11.5	
P19	P20	30	98.28	90.1	8.2	
P21	P22	30	102.5	91.0	11.5	
P23	P24	30	103.2	91.7	11.5	
P25	P26	30	103.9	92.4	11.5	
P27	P28	30	104.3	92.8	11.5	
P29	P30	30	105.1	93.6	11.5	
P31	P32	30	105.3	93.8	11.5	
P33	P34	30	105.9	94.4	11.5	
P35	P36	30	106.5	95.0	11.5	
P37	P38	30	107.1	95.6	11.5	

1) TOP OF PLUG SHAFT CONCRETE ELEVATION IS 3" BELOW THE APPROXIMATE GRADES AT EDGE

2) ACTUAL PLUG SHAFT LENGTH TO BE DETERMINED BY CONDITIONS ENCOUNTERED IN FIELD. PLUG SHAFTS SHOULD EXTEND A MINIMUM OF 6" INTO TOP OF UNWEATHERED (GRAY) BEDROCK.

#### **DRILLED SHAFT CONSTRUCTION NOTES**

#### **DRILLED SHAFT INSTALLATION**

- 1. CONSTRUCT THE RETAINING WALL CONSISTING OF 24-INCH DIAMETER DRILLED STRUCTURAL SHAFTS WITH PLUG SHAFT LAGGING USING ROLLED STEEL SECTIONS OR STEEL CAGE REINFORCEMENT AS SHOWN AND DESCRIBED ON PLANS. THIS PURPOSE OF THIS WORK IS TO STOP LATERAL CREEP-TYPE MOVEMENT ON THE DOWNSLOPE (NORTH) SIDE OF OLIVE BRANCH ROAD. CONTRACTOR IS RESPONSIBLE FOR CLEARING OVERHEAD AND UNDERGROUND UTILITIES AND PROVIDING ACCESS FOR EQUIPMENT. THE SHAFT EXCAVATION SHALL BE UNCLASSIFIED.
- 2. THE REINFORCED DRILLED SHAFT RETAINING WALL WILL CONSIST OF DRILLED SHAFTS SPACED APPROXIMATELY ON 6 FEET -3 INCHES (6'-3") ON CENTER, AS SHOWN ON THE SITE PLAN DRAWING. ESTIMATED EMBEDMENT DEPTHS INTO INTERBEDDED SHALE AND LIMESTONE BEDROCK ARE SHOWN ON THE DRILLED SHAFT SCHEDULE TABLE ON THIS SHEET; HOWEVER, ACTUAL EMBEDMENT DEPTHS WILL BE BASED ON ACTUAL FIELD CONDITIONS AS DETERMINED BY THE GEOTECHNICAL CONSULTANT.
- 3. THE SHAFTS SHALL BE LOCATED AS SHOWN ON PLAN WITHIN 6" OF PLAN LOCATION. THE SHAFTS SHALL MAINTAIN A PLUMBNESS DEVIATION OF A MAXIMUM OF 1" IN 12 FT. VERTICAL HEIGHT. A MINIMUM 3" CONCRETE COVER BETWEEN THE REINFORCING STEEL AND THE EXTERIOR (SIDES AND TOP) OF THE DRILLED SHAFT SHALL BE PROVIDED.
- 4. TWO ALTERNATIVES FOR REINFORCEMENT OF STRUCTURAL SHAFTS HAVE BEEN PROVIDED. REINFORCEMENT FOR THE STRUCTURAL SHAFTS SHALL CONSIST OF EITHER ROLLED STEEL SECTIONS HAVING YIELD STRENGTH OF 50 KSI OR CAGE REINFORCING STEEL HAVING A YIELD STRENGTH OF 60 KSI. ROLLED STEEL SECTIONS (ALTERNATIVE 1) SHOULD CONSIST OF HP 12X53, AS NOTED ON THE PLANS. THE CONFIGURATION OF THE STEEL REINFORCING CAGE (ALTERNATIVE 2) HAS BEEN SHOWN ON THE PLANS.
- 5. THE PRE-DRILLED SHAFT WILL BE BACKFILLED WITH CONCRETE TO THE TOP OF CONCRETE ELEVATION. THE TOP OF SHAFT CONCRETE ELEVATION SHALL BE ABOUT 3 INCHES BELOW THE APPROXIMATE EXISTING GRADE AT THE CENTERLINE OF THE PROPOSED DRILLED SHAFTS, TO AVOID CONTACT BY SNOW PLOWS. STRUCTURE SHAFT CONCRETE SHALL BE CONCRETE (28-DAY F'C = 4000 PSI, MAXIMUM SLUMP = 6 INCHES; 4% TO 6% AIR ENTRAINMENT) PLACED USING FREE FALL METHOD OF PLACEMENT. CONCRETE SHALL BE PLACED INTO EACH SHAFT EXCAVATION ON THE SAME DAY THAT THE DRILLING IS COMPLETED. SEE ITEM 9 BELOW.
- 6. TEMPORARY STEEL CASING SHOULD BE ON-SITE AND USED WHEREVER REQUIRED TO STABILIZE LOOSE OR CAVING MATERIALS, OR TO SEAL OFF WATER BEARING ZONES ENCOUNTERED DURING CONSTRUCTION.
- 7. THE WALL DESIGN IS BASED ON MAXIMUM DEPTH TO UNWEATHERED (GRAY) INTERBEDDED SHALE AND LIMESTONE BEDROCK ABOUT 11 FEET BELOW GRADE AS NOTED IN THE DRILLED SHAFT SCHEDULE. HOWEVER, IF BEDROCK IS ENCOUNTERED MORE THAN 1.0 FT. BELOW PLAN DESIGN ELEVATION, TERRACON SHALL BE NOTIFIED IMMEDIATELY TO REVIEW AND PROVIDE ADDITIONAL RECOMMENDATIONS. EACH SHAFT SHALL BE SOCKETED INTO BEDROCK A MINIMUM OF 9 FEET, AS DESCRIBED ON THE PLANS.
- 8. THE CONTRACTOR SHALL MAINTAIN A RECORD OF EACH SHAFT DRILLED, WHICH WILL INCLUDE AT A MINIMUM: SHAFT NUMBER; GROUND ELEVATION; SHAFT TOP ELEVATION; TOP OF UNWEATHERED (GRAY) BEDROCK ELEVATIONS; AS-BUILT ROCK SOCKET DEPTH; ELEVATION OF THE TOP OF THE SHAFT CONCRETE; DATE DRILLED; DATE COMPLETED; AND WEATHER CONDITIONS.
- 9. IT IS ANTICIPATED THAT WATER MAY ENTER SOME OF THE SHAFT EXCAVATIONS. THE DEPTH OF PONDED WATER AT THE BOTTOM OF THE SHAFT EXCAVATIONS SHOULD NOT EXCEED 2 INCHES, PRIOR TO PLACING CONCRETE. IF THE WATER CANNOT BE PUMPED DOWN, TREMIE PLACEMENT METHODS WILL BE REQUIRED.
- 10. THE DRILLED SHAFT EXCAVATIONS SHOULD BE INSPECTED BY A QUALIFIED GEOTECHNICAL REPRESENTATIVE TO CONFIRM THAT THE DRILLED SHAFTS ARE SOCKETED INTO BEDROCK ACCORDING TO DESIGN, AND THAT THE DRILLED SHAFTS HAVE BEEN CONSTRUCTED PER SPECIFICATIONS.
- 11. SHAFT SPOILS SHALL BE TRUCKED FROM THE SITE (NOT WASTED ON THE HILLSIDE). NO FILL PLACEMENT SHOULD BE ALLOWED DOWNSLOPE OF THE SLOPE FACE.

#### PLUG SHAFT INSTALLATION

- 1. TWO 30-INCH DIAMETER PLUG SHAFTS CONSISTING OF UNREINFORCED CONCRETE (28-DAY  $f_C = 2500$  PSI, MAXIMUM SLUMP = 6 INCHES; 4% TO 6% AIR ENTRAINMENT) AS NOTED ON THE CROSS-SECTION DETAILS WILL ACT AS LAGGING FOR THE SHAFT WALL.
- 2. PLUG SHAFT INSTALLATION FOR THE DRILLED SHAFT RETAINING WALL SHALL BEGIN AFTER THE STRUCTURAL SHAFT ELEMENTS HAVE GAINED STRENGTH (AT LEAST 24 HOURS AFTER PLACEMENT OF STRUCTURAL SHAFT CONCRETE).
- 3. THE TOP OF THE PLUG SHAFT CONCRETE SHALL BE ABOUT 3 INCHES BELOW THE PAVEMENT.
- 4. THE BOTTOM OF ALL THE PLUG SHAFTS INSTALLED SHOULD EXTEND TO THE ELEVATIONS DETAILED IN THE DRILLED SHAFT SCHEDULE (A MINIMUM OF 6 INCHES BELOW TOP OF UNWEATHERED BEDROCK).
- 5. PLUG SHAFT SPOILS SHALL BE TRUCKED FROM THE SITE (NOT WASTED ON THE HILLSIDE).

#### DRAINAGE AND OTHER CONSTRUCTION CONSIDERATIONS

1. NEW STRUCTURAL FILL MAY BE PLACED ON UPSLOPE SIDE OF SHAFTS TO RETAIN GRADE NEXT TO THE EDGE OF PAVEMENT. SONOTUBES OR EQUIVALENT WILL BE REQUIRED IN SOME AREAS, DUE TO THE STEEPLY SLOPING HILLSIDE BELOW THE WALL. FILL SHOULD BE PLACED AND COMPACTED PER ODOT SPECIFICATIONS (ITEM 203). ONLY HAND-OPERATED EQUIPMENT SHOULD BE USED WITHIN 5 FEET OF THE FRONT OF THE SHAFTS.

#### FIELD QUALITY CONTROL

A. OWNER WILL COORDINATE FIELD CONSTRUCTION INSPECTION AND REPORTING THROUGH IN-HOUSE PERSONNEL OR EXTERNAL TESTING AGENCY.

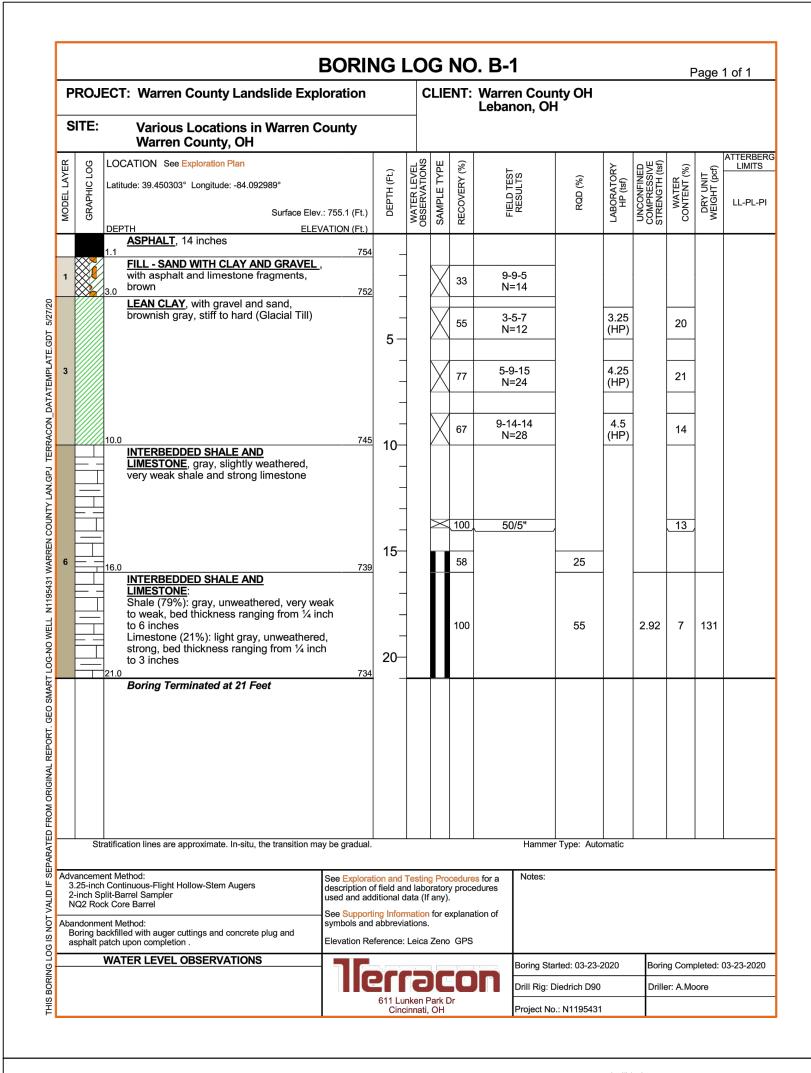
DOCUMENTATION SHALL INCLUDE THE FOLLOWING AT EACH DRILLED SHAFT:

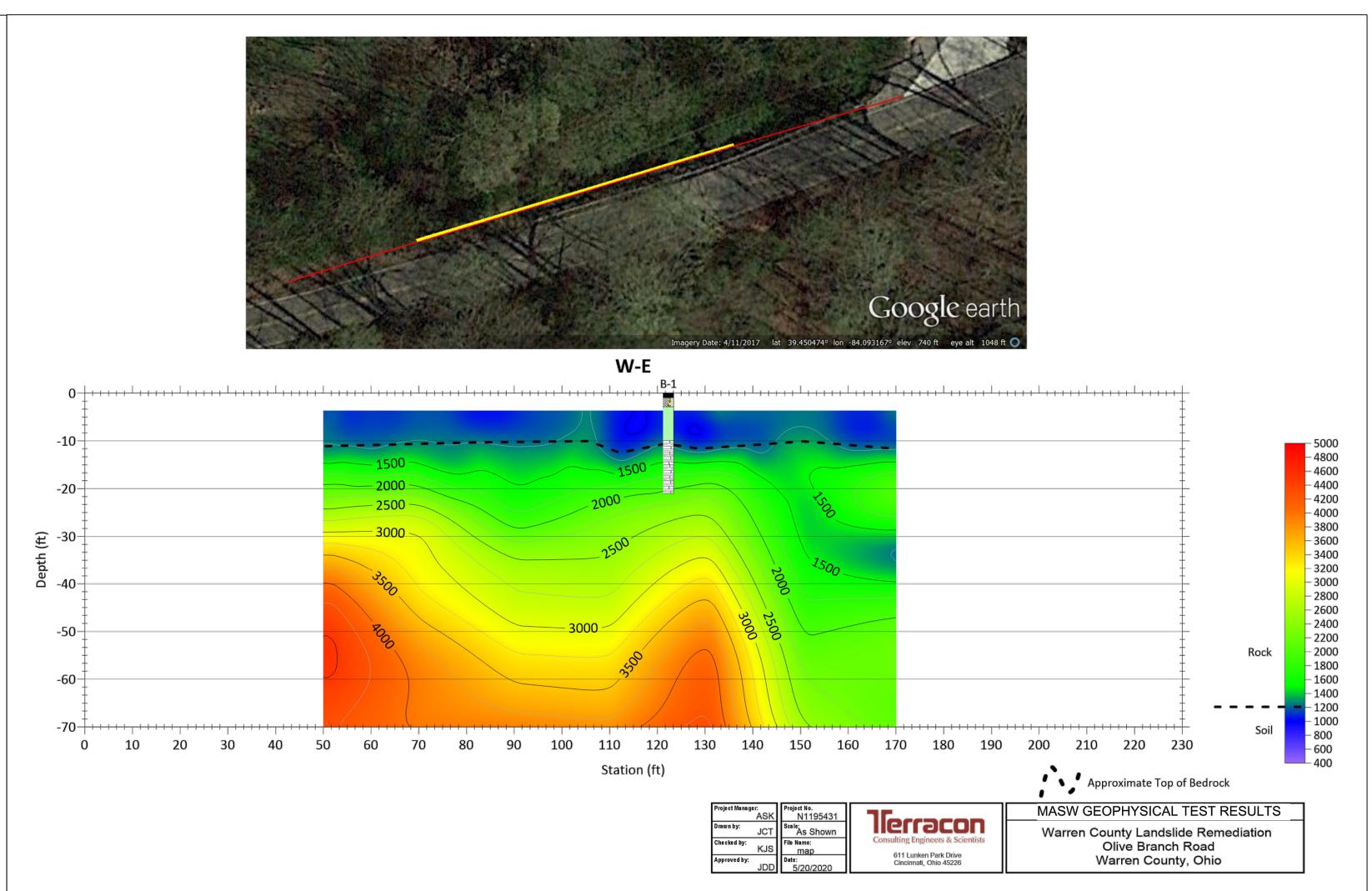
- GROUND ELEVATION
- 2. AS-BUILT SHAFT DIAMETER AND TOP AND BOTTOM SHAFT ELEVATIONS.
- 3. TOP OF WEATHERED (IF ANY) AND UNWEATHERED (GRAY) BEDROCK ELEVATION.
- 4. DESCRIPTION OF ENCOUNTERED SOIL MATERIALS. 5. DESCRIPTION, LOCATION, AND DIMENSIONS OF OBSTRUCTIONS.
- FINAL TOP CENTERLINE LOCATION AND DEVIATIONS FROM REQUIREMENTS.
- 7. VARIATION OF SHAFT FROM PLUMB.
- 8. DRILLED SHAFT EXCAVATING METHOD.
- 9. LENGTH OF ROCK SOCKET.
- 10.LEVELNESS OF SHAFT BOTTOM AND ADEQUACY OF CLEANOUT.
- 11. GROUND-WATER CONDITIONS AND WATER-INFILTRATION RATE, DEPTH, AND PUMPING. 12. DESCRIPTION, DIAMETER, AND TOP AND BOTTOM ELEVATIONS OF TEMPORARY OR PERMANENT CASINGS.
- 13.DESCRIPTION OF SOIL OR WATER MOVEMENT, SIDEWALL STABILITY, LOSS OF GROUND, AND MEANS OF
- 14. DATE AND TIME OF STARTING AND COMPLETING DRILLED SHAFT EXCAVATION.
- 15. POSITION OF REINFORCING STEEL.
- 16. CONCRETE PLACEMENT METHOD, INCLUDING DELAYS.
- 17. ELEVATION OF CONCRETE DURING REMOVAL OF CASINGS.
- 18.LOCATIONS OF CONSTRUCTION JOINTS, IF ANY.
- 19. REMARKS, UNUSUAL CONDITIONS ENCOUNTERED, AND DEVIATIONS FROM REQUIREMENTS.

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SHEET 3				
DESIGNED BY:	ASK/DWW			
DRAWN BY:	KM			
APPVD. BY:	JDD			
SCALE:	AS SHOWN			
DATE:	07/29/2020			
JOB NO.	N1205222			
ACAD NO.	WC RW.DWG			
SHEET NO.:	3			





#### **GENERAL NOTES**



SAMPLING	
Rock Core Standard Penetration Test  Split Spoon	

#### DESCRIPTIVE SOIL CLASSIFICATION

Soil classification is based on the Unified Soil Classification System. Coarse Grained Soils have more than 50% of their dry weight retained on a #200 sieve; their principal descriptors are: boulders, cobbles, gravel or sand. Fine Grained Soils have less than 50% of their dry weight retained on a #200 sieve; they are principally described as clays if they are plastic, and silts if they are slightly plastic or non-plastic. Major constituents may be added as modifiers and minor constituents may be added according to the relative proportions based on grain size. In addition to gradation, coarse-grained soils are defined on the basis of their in-place relative density and fine-grained soils on the basis of their consistency.

#### LOCATION AND ELEVATION NOTES

Unless otherwise noted, Latitude and Longitude are approximately determined using a hand-held GPS device. The accuracy of such devices is variable. Surface elevation data annotated with +/- indicates that no actual topographical survey was conducted to confirm the surface elevation. Instead, the surface elevation was approximately determined from topographic maps of the area.

	STRENGTH TERMS					
RELATIVE DENSITY	RELATIVE DENSITY OF COARSE-GRAINED SOILS CONSISTENCY OF FINE-GRAINED SOILS					
	(More than 50% retained on No. 200 sieve.) Density determined by Standard Penetration Resistance		(50% or more passing the No. 200 stermined by laboratory shear strength to procedures or standard penetration re	esting, field visual-manual		
Descriptive Term (Density)	Standard Penetration or N-Value Blows/Ft.	Descriptive Term (Consistency)	Unconfined Compressive Strength Qu, (tsf)	Standard Penetration or N-Value Blows/Ft.		
Very Loose	0 - 3	Very Soft	less than 0.25	0 - 1		
Loose	4 - 9	Soft	0.25 to 0.50	2 - 4		
Medium Dense	10 - 29	Medium Stiff	0.50 to 1.00	4 - 8		
Dense	30 - 50	Stiff	1.00 to 2.00	8 - 15		
Very Dense	> 50	Very Stiff	2.00 to 4.00	15 - 30		
		Hard	> 4.00	> 30		

RELATIVE PROPORTION	S OF SAND AND GRAVEL	RELATIVE PROPO	RTIONS OF FINES
Descriptive Term(s) of other constituents	Percent of Descriptive Term(s) of Other constituents		Percent of Dry Weight
Trace	<15	Trace	<5
With	15-29	With	5-12
Modifier	>30	Modifier	>12
GRAIN SIZE T	ERMINOLOGY	PLASTICITY [	DESCRIPTION
Major Component of Sample	Particle Size	Term	Plasticity Index
Boulders	Over 12 in. (300 mm)	Non-plastic	0
Cobbles	12 in. to 3 in. (300mm to 75mm)	Low	1 - 10
Gravel	3 in. to #4 sieve (75mm to 4.75 mm)	Medium	11 - 30
Sand	#4 to #200 sieve (4.75mm to 0.075mm	High	> 30
Silt or Clay	Passing #200 sieve (0.075mm)		

Terracon

					S	oil Classification
Criteria for Assigning Group Symbols and Group Names Using Laboratory Tests A						Group Name <sup>B</sup>
		Clean Gravels:			GW	Well-graded gravel F
	<b>Gravels:</b> More than 50% of	Less than 5% fines <sup>C</sup>	Cu < 4 and/or [Cc<1 or Cc>3.0] E		GP	Poorly graded gravel F
	coarse fraction retained on No. 4 sieve	Gravels with Fines:	Fines classify as ML or MH		GM	Silty gravel F, G, H
Coarse-Grained Soils:	retained on No. 4 sieve	More than 12% fines •	Fines classify as CL or CH		GC	Clayey gravel F, G, H
More than 50% retained on No. 200 sieve	Sands: 50% or more of coarse fraction passes No. 4 sieve	Clean Sands:	Cu ≥ 6 and 1 ≤ Cc ≤ 3 E		SW	Well-graded sand
		Less than 5% fines D	Cu < 6 and/or [Cc<1 or Cc>3.	.0] ■	SP	Poorly graded sand
		Sands with Fines:	Fines classify as ML or MH		SM	Silty sand G, H, I
		More than 12% fines D	Fines classify as CL or CH		sc	Clayey sand <sup>G, H, I</sup>
		Inorganic:	PI > 7 and plots on or above "A"		CL	Lean clay K, L, M
	Silts and Clays:		PI < 4 or plots below "A" line   J		ML	Silt K, L, M
	Liquid limit less than 50	Organia	Liquid limit - oven dried	0.75	75 OL	Organic clay K, L, M, N
Fine-Grained Soils:		Organic:	Liquid limit - not dried	0.75		Organic silt K, L, M, O
50% or more passes the No. 200 sieve		Inorganic:	PI plots on or above "A" line		СН	Fat clay <b>K, ∟, M</b>
	Silts and Clays:	morganic.	PI plots below "A" line		МН	Elastic Silt K, L, M
	Liquid limit 50 or more	Ormonia	Liquid limit - oven dried	< 0.75 OH	Organic clay K, L, M, P	
		Organic:	Liquid limit - not dried	0.75		Organic silt K, L, M, Q
Highly organic soils:	Primarily	organic matter, dark in co	olor, and organic odor		PT	Peat

ABased on the material passing the 3-inch (75-mm) sieve. <sup>B</sup> If field sample contained cobbles or boulders, or both, add "with cobbles or boulders, or both" to group name. Gravels with 5 to 12% fines require dual symbols: GW-GM well-graded gravel with silt, GW-GC well-graded gravel with clay, GP-GM poorly graded gravel with silt, GP-GC poorly graded gravel with clay. Sands with 5 to 12% fines require dual symbols: SW-SM well-graded

sand with silt, SW-SC well-graded sand with clay, SP-SM poorly graded sand with silt, SP-SC poorly graded sand with clay.  $E Cu = D_{60}/D_{10} \quad Cc =$ 

F If soil contains ≥ 15% sand, add "with sand" to group name.

<sup>6</sup> If fines classify as CL-ML, use dual symbol GC-GM, or SC-SM.

NPI ≥ 4 and plots on or above "A" line. OPI < 4 or plots below "A" line.</p> PI plots on or above "A" line. PI plots below "A" line.

"sandy" to group name.

"gravelly" to group name.

gravel," whichever is predominant.

HIf fines are organic, add "with organic fines" to group name.

If soil contains ≥ 15% gravel, add "with gravel" to group name.

If Atterberg limits plot in shaded area, soil is a CL-ML, silty clay.

KIf soil contains 15 to 29% plus No. 200, add "with sand" or "with

Left soil contains ≥ 30% plus No. 200 predominantly sand, add

MIf soil contains ≥ 30% plus No. 200, predominantly gravel, add

For classification of fine-grained soils and fine-grained fraction <sub>50</sub> —of coarse-grained soils Equation of "A" - line Horizontal at PI=4 to LL=25.5. 40 - then PI=0.73 (LL-20) Equation of "U" - line Vertical at LL=16 to PI=7, 30 then PI=0.9 (LL-8)

LIQUID LIMIT (LL)

#### DESCRIPTION OF ROCK PROPERTIES

lerracon

	WEATHERING
Term	Description
Unweathered	No visible sign of rock material weathering, perhaps slight discoloration on major discontinuity surfaces.
Slightly weathered	Discoloration indicates weathering of rock material and discontinuity surfaces. All the rock material may be discolored by weathering and may be somewhat weaker externally than in its fresh condition.
Moderately weathered	Less than half of the rock material is decomposed and/or disintegrated to a soil. Fresh or discolored rock is present either as a continuous framework or as corestones.
Highly weathered	More than half of the rock material is decomposed and/or disintegrated to a soil. Fresh or discolored rock is present either as a discontinuous framework or as corestones.
Completely weathered	All rock material is decomposed and/or disintegrated to soil. The original mass structure is still largely intact.
Residual soil	All rock material is converted to soil. The mass structure and material fabric are destroyed. There is a large change in volume, but the soil has not been significantly transported.
	STRENGTH OR HARDNESS

Description	Field Identification	Strength, psi (MPa)		
Extremely weak	Indented by thumbnail	40-150 (0.3-1)		
Very weak	Crumbles under firm blows with point of geological hammer, can be peeled by a pocket knife	150-700 (1-5)		
Weak rock	Can be peeled by a pocket knife with difficulty, shallow indentations made by firm blow with point of geological hammer	700-4,000 (5-30)		
Medium strong	Cannot be scraped or peeled with a pocket knife, specimen can be fractured with single firm blow of geological hammer	4,000-7,000 (30-50)		
Strong rock	Specimen requires more than one blow of geological hammer to fracture it	7,000-15,000 (50-100)		
Very strong	Specimen requires many blows of geological hammer to fracture it	15,000-36,000 (100-250)		
Extremely strong	Specimen can only be chipped with geological hammer	>36,000 (>250)		
DISCONTINUITY DESCRIPTION				

DISCONTINUITY DESCRIPTION				
Fracture Spacing (Joints, Faults, Other Fractures)		Bedding Spacing (May Include Foliation or Banding)		
Description	Spacing	Description	Spacing	
Extremely close	< ¾ in (<19 mm)	Laminated	< ½ in (<12 mm)	
Very close	¾ in – 2-1/2 in (19 - 60 mm)	Very thin	½ in – 2 in (12 – 50 mm)	
Close	2-1/2 in – 8 in (60 – 200 mm)	Thin	2 in – 1 ft. (50 – 300 mm)	
Moderate	8 in – 2 ft. (200 – 600 mm)	Medium	1 ft. – 3 ft. (300 – 900 mm)	
Wide	2 ft. – 6 ft. (600 mm – 2.0 m)	Thick	3 ft. – 10 ft. (900 mm – 3 m)	
Very Wide	6 ft. – 20 ft. (2.0 – 6 m)	Massive	> 10 ft. (3 m)	

<u>Discontinuity Orientation (Angle)</u>: Measure the angle of discontinuity relative to a plane perpendicular to the longitudinal axis of the core. (For most cases, the core axis is vertical; therefore, the plane perpendicular to the core axis is horizontal.) For example, a horizontal bedding plane would have a 0-degree angle.

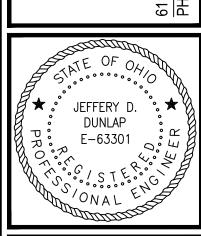
ROCK QUALITY DESIGNATION (RQD) 1			
Description	RQD Value (%)		
Very Poor	0 - 25		
Poor	25 – 50		
Fair	50 – 75		
Good	75 – 90		
Excellent	90 - 100		

1. The combined length of all sound and intact core segments equal to or greater than 4 inches in length, expressed as a percentage of the total core run length.

Reference: U.S. Department of Transportation, Federal Highway Administration, Publication No FHWA-NHI-10-034, December 2009

<u>Technical Manual for Design and Construction of Road Tunnels – Civil Elements</u>

ROAD (CR 217) LANDSLIDE REMEDIATION OUNTY ENGINEER'S OFFICE WARREN COUNTY, OHIO E C E BRANCH



SHEET 4 DESIGNED BY: | ASK/DWW DRAWN BY: KM APPVD. BY: JDD SCALE: AS SHOWN 07/29/2020 N1205222 WC RW.DWG JOB NO. ACAD NO. SHEET NO.:

